

STATE PROJECT NUMBER

* DIMENSION IS TAKEN NORMAL TO \P SUBSTRUCTURE UNITS.

* * DIMENSION IS TAKEN PARALLEL TO & GIRDER.

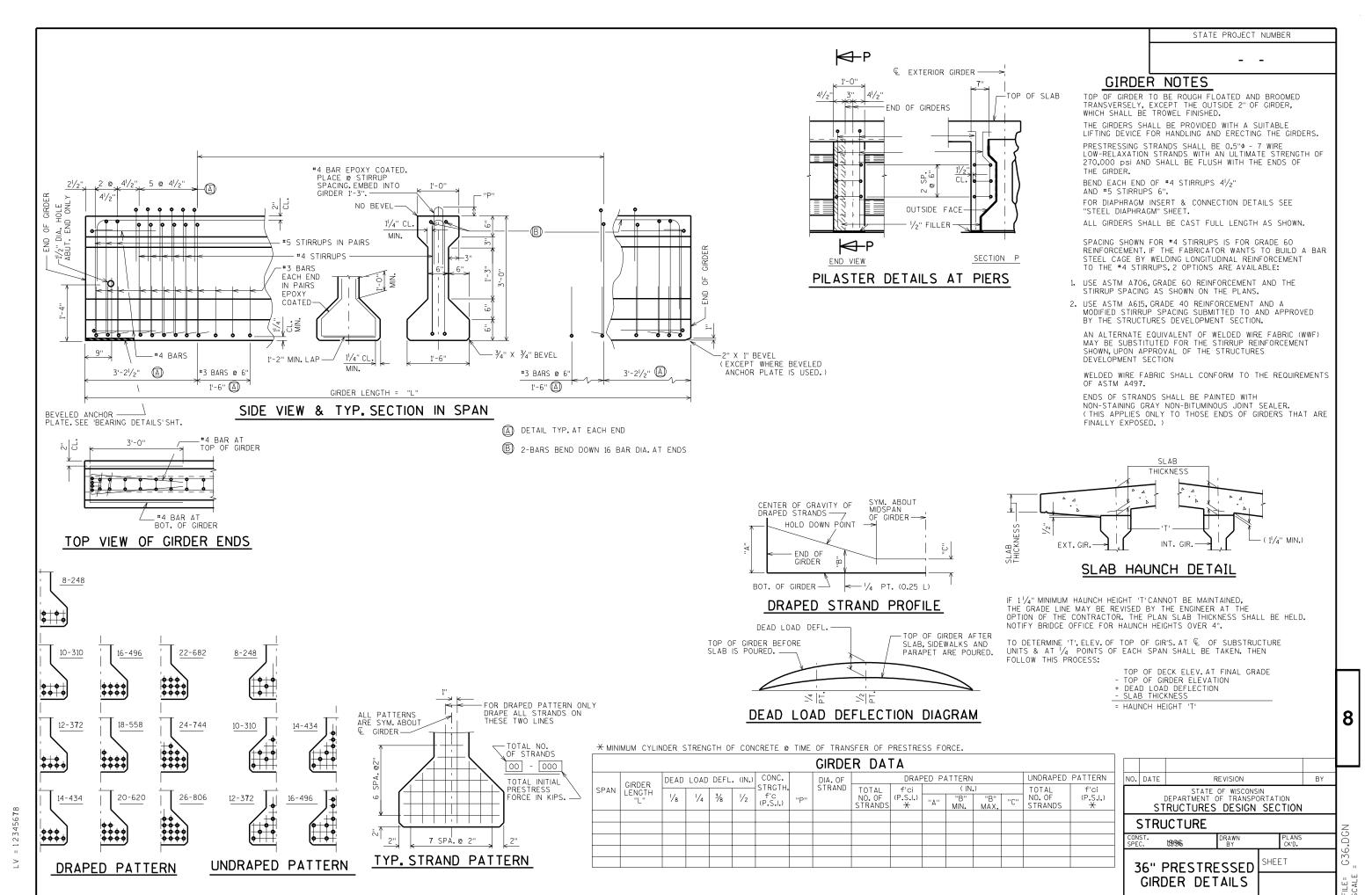
PART LONGIT. SECTION

NO. DATE

STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE 1996

28" PRESTRESSED SHEET GIRDER DETAILS



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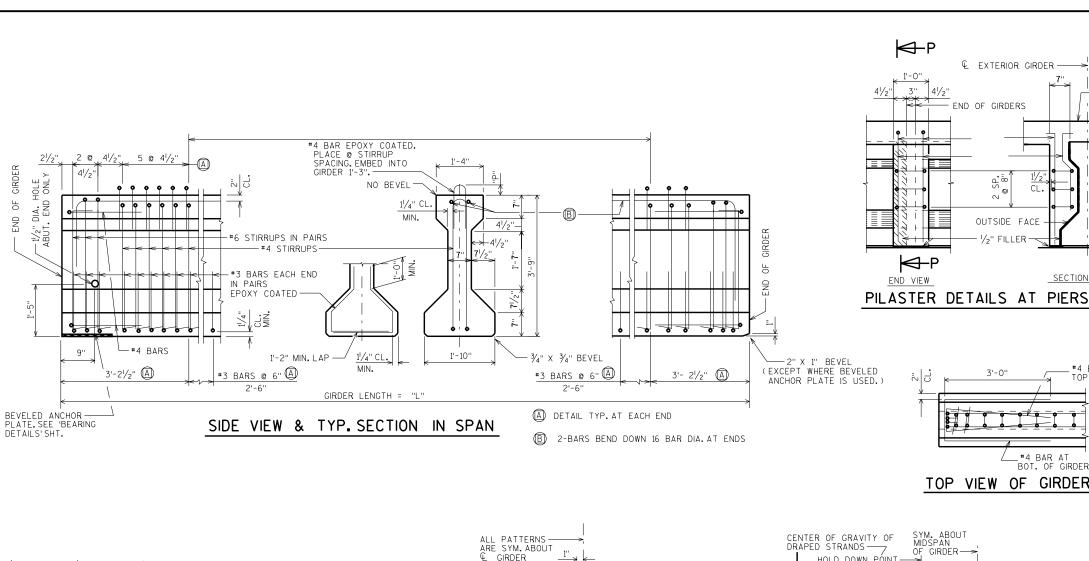
** DIMENSION IS TAKEN PARALLEL TO Q GIRDER.

PART LONGIT. SECTION

NO. DATE STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

SUPERSTRUCTURE SHEET DETAILS



24-**7**44

UNDRAPED PATTERN

18-558

GIRDER NOTES

TOP OF GIRDER TO BE ROUGH FLOATED AND BROOMED TRANSVERSELY, EXCEPT THE OUTSIDE 2" OF GIRDER, WHICH SHALL BE TROWEL FINISHED.

THE GIRDERS SHALL BE PROVIDED WITH A SUITABLE LIFTING DEVICE FOR HANDLING AND ERECTING THE GIRDERS.

STATE PROJECT NUMBER

PRESTRESSING STRANDS SHALL BE 0.5" \$\phi\$ - 7 WIRE LOW-RELAXATION STRANDS WITH AN ULTIMATE STRENGTH OF 270,000 psi AND SHALL BE FLUSH WITH THE ENDS OF THE GIRDER.

BEND EACH END OF #4 STIRRUPS 41/2 AND #6 STIRRUPS 61/2".

FOR DIAPHRAGM INSERT & CONNECTION DETAILS SEE "STEEL DIAPHRAGM" SHEET.

ALL GIRDERS SHALL BE CAST FULL LENGTH AS SHOWN.

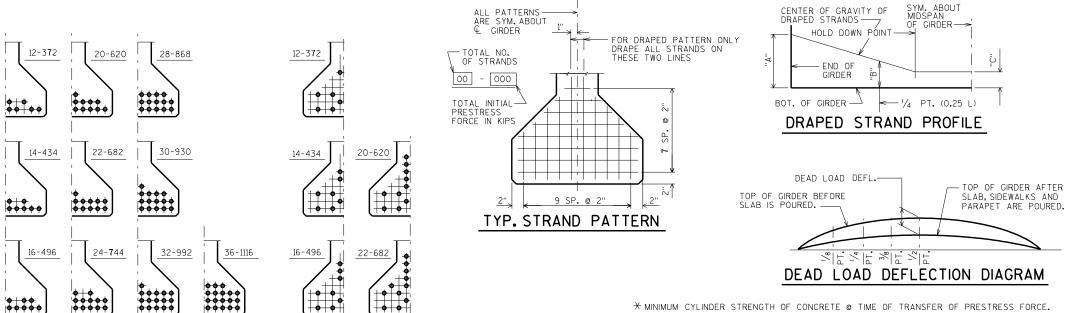
SPACING SHOWN FOR #4 STIRRUPS IS FOR GRADE 60 REINFORCEMENT, IF THE FABRICATOR WANTS TO BUILD A BAR STEEL CAGE BY WELDING LONGITUDINAL REINFORCEMENT TO THE #4 STIRRUPS, 2 OPTIONS ARE AVAILABLE:

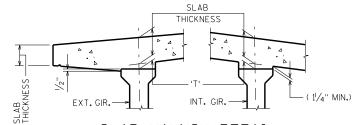
- 1. USE ASTM A706, GRADE 60 REINFORCEMENT AND THE STIRRUP SPACING AS SHOWN ON THE PLANS.
- 2. USE ASTM A615, GRADE 40 REINFORCEMENT AND A MODIFIED STIRRUP SPACING SUBMITTED TO AND APPROVED BY THE STRUCTURES DEVELOPMENT SECTION.

AN ALTERNATE EQUIVALENT OF WELDED WIRE FABRIC (WWF) MAY BE SUBSTITUTED FOR THE STIRRUP REINFORCEMENT SHOWN, UPON APPROVAL OF THE STRUCTURES DEVELOPMENT SECTION

WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A497.

ENDS OF STRANDS SHALL BE PAINTED WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER. (THIS APPLIES ONLY TO THOSE ENDS OF GIRDERS THAT ARE FINALLY EXPOSED.)





SLAB HAUNCH DETAIL

IF $1^1/\!\!/_4$ " MINIMUM HAUNCH HEIGHT AT EDGE OF GIRDER CANNOT BE MAINTAINED, THE GRADE LINE MAY BE REVISED BY THE ENGINEER AT THE OPTION OF THE CONTRACTOR, THE PLAN SLAB THICKNESS SHALL BE HELD. NOTIFY BRIDGE

TO DETERMINE 'T', ELEV. OF TOP OF GIR'S. AT $\mathbb Q$ OF SUBSTRUCTURE UNITS & AT $\mathcal I_8$ POINTS OF EACH SPAN SHALL BE TAKEN. THEN FOLLOW THIS PROCESS:

- TOP OF DECK ELEV. AT FINAL GRADE
 TOP OF GIRDER ELEVATION
 + DEAD LOAD DEFLECTION
 SLAB THICKNESS

- = HAUNCH HEIGHT 'T'

IMUM CYLI	NDER S	TREN	STH O	F CON	CRETE @	TIME				SS FO	ORCE.					1				
							GIRDE	R DAT	ΓΑ											
0.0050	DEAD	LOAD	DEFL	. (IN.)	CONC.		DIA, OF		DRAF	PED P.	ATTERN			UNDRAPED	PATTERN	NO. DATE		REVISION		BY
LENGTH "L"	1/8	1/4	3/8	1/2	STRGTH. f'c (P.S.I.)	"P"	STRAND	TOTAL NO. OF STRANDS	f'ci (P.S.I.) X	"A"	"B"	"B"	"C"	TOTAL NO. OF STRANDS	f'ci (P.S.I.) X		DEPARTMENT	OF TRANSPO	ORTATION	
																STRU	ICTURE			
																CONST. SPEC.	1996	DRAWN BY	PLANS CK'D.	
																45" PI	RESTRE	SSED	SHEET	
•												'				GIRDI	ER DET	AILS		
	GIRDER LENGTH	GIRDER DEAD	GIRDER DEAD LOAD	GIRDER DEAD LOAD DEFL	GIRDER DEAD LOAD DEFL. (IN.)	GIRDER LOAD DEFL. (IN.) CONC. STRGTH.	GIRDER LENGTH L/A 1/A 3/A 1/A 1/A 1/A 1/A 1/A 1/A 1/A 1/A 1/A 1	GIRDER GIRDER LENGTH 1/2 1/3 1/4 1/4 1/4 1/4 1/4 1/4 1/4	GIRDER DATE OF STRAND TOTAL NO. OF	GIRDER DATA GIRDER LENGTH 1/8 1/4 3/8 1/2 1/5	GIRDER DATA GIRDER LENGTH 1/8 1/4 3/8 1/2 1/5 1/6	GIRDER LENGTH	GIRDER DATA GIRDER DEAD LOAD DEFL. (IN.) CONC. STRGTH. STRAND TOTAL F'CI (IN.) (IN.	GIRDER DATA GIRDER DEAD LOAD DEFL. (IN.) CONC. STRGTH. Fici CINC. NO. OF (P.S.I.) (IN.) (GIRDER DATA CONC. STRGTH. STR	GIRDER DATA GIRDER DEAD LOAD DEFL. (IN.) CONC. STRGTH. F'ci CIN. TOTAL F'ci CIN. TOTAL F'ci CIN. TOTAL CIN. TO	GIRDER DATA GIRDER DATA DEAD LOAD DEFL. (IN.) CONC. STRGTH, F'C (P.S.I.) F'C (P.S.I.) F'C (P.S.I.) STRANDS F'DIA. OF STRANDS F'DI	GIRDER DATA GIRDER DATA DEAD LOAD DEFL. (IN.) CONC. STRGTH, F'C (P.S.I.) F'C (P.S.I.) F'C (P.S.I.) TOTAL NO. OF STRANDS TO	GIRDER DATA GIRDER DATA CONC. CONC.	CIRDER DATA

EXTERIOR GIRDER -

CL

_#4 BAR AT

TOP VIEW OF GIRDER ENDS

BOT. OF GIRDER

SECTION P

#4 BAR AT TOP OF GIRDER

OUTSIDE FACE

END OF GIRDERS

TOP OF SLAB

G45.DGN

8

= 12345678

26-806

DRAPED PATTERN

18-558

0000

34-1054

38-11**7**8

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PART LONGIT. SECTION

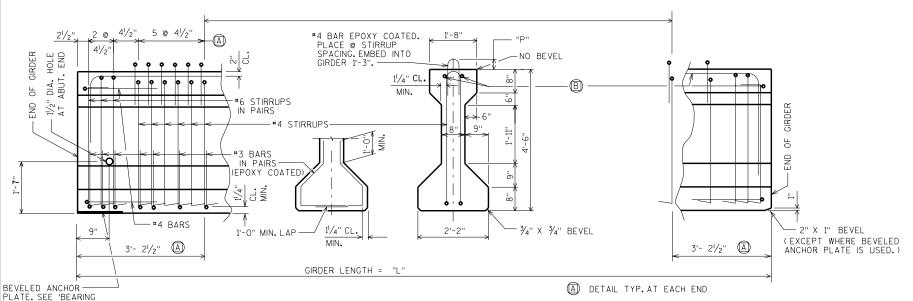
NO. DATE REVISION BY

STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

DNST. PEC. 1996

SUPERSTRUCTURE DETAILS



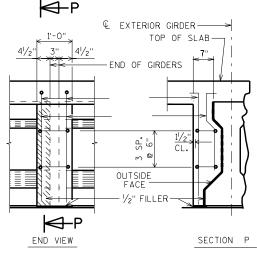
ALL PATTERNS -

ARE SYM. ABOUT

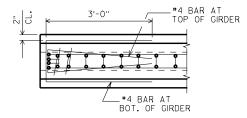
€ GIRDER

SIDE VIEW & TYP. SECTION IN SPAN

UNDRAPED PATTERN



PILASTER DETAILS AT PIERS



TOP VIEW OF GIRDER ENDS

GIRDER NOTES

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PRESTRESSING STRANDS SHALL BE 0.5" 4 - 7 WIRE LOW-RELAXATION STRANDS WITH AN ULTIMATE STRENGTH OF 270,000 psi AND SHALL BE FLUSH WITH THE ENDS OF THE GIRDER.

BEND EACH END OF #4 STIRRUPS 41/2" AND #6 STIRRUPS 61/2".

FOR DIAPHRAGM INSERT & CONNECTION DETAILS SEE "STEEL DIAPHRAGM" SHEET.

ALL GIRDERS SHALL BE CAST FULL LENGTH AS SHOWN.

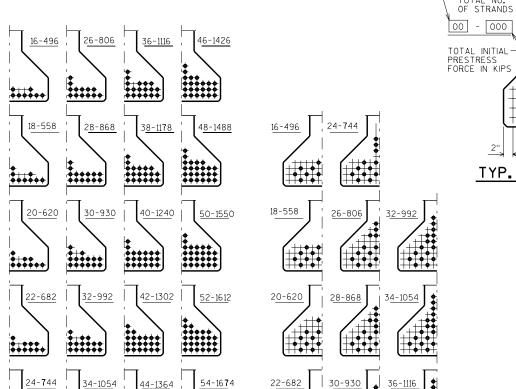
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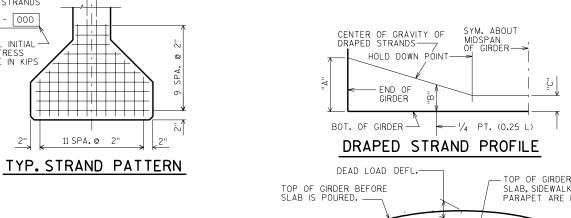
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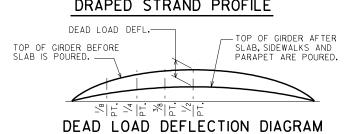


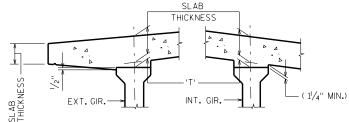


(B) 2-BARS BEND DOWN 16 BAR DIA. AT ENDS

FOR DRAPED PATTERN ONLY

DRAPE ALL STRANDS ON THESE TWO LINES





SLAB HAUNCH DETAIL

IF $1^1/4$ " MINIMUM HAUNCH HEIGHT AT EDGE OF GIRDER CANNOT BE MAINTAINED, THE GRADE LINE MAY BE REVISED BY THE ENGINEER AT THE OPTION OF THE CONTRACTOR, THE PLAN SLAB THICKNESS SHALL BE HELD, NOTIFY BRIDGE OFFICE FOR HAUNCH HEIGHTS OVER 4".

TO DETERMINE 'T', ELEV. OF TOP OF GIR'S, AT \P . OF SUBSTRUCTURE UNITS & AT 1/8 POINTS OF EACH SPAN SHALL BE TAKEN. THEN FOLLOW THIS PROCESS:

- TOP OF DECK ELEV. AT FINAL GRADE TOP OF GIRDER ELEVATION
- SLAB THICKNESS
- = HAUNCH HEIGHT 'T

								GIRDE	R DA	ΓΑ						
	CIDDED	DEAD	LOAD	DEFL	. (IN.)	CONC.				DRAF	PED P.	ATTERN			UNDRAPED	PATTERN
SPAN	GIRDER LENGTH "L"	1/8	1/4	3/8	1/2	STRGTH. f'c (P.S.I.)	"P"	DIA. OF STRAND	TOTAL NO. OF STRANDS	f'ci (P.S.I.) X	"A"	(IN. "B" MIN.	.) "B" MAX.	"C"	TOTAL NO.OF STRANDS	f'ci (P.S.I.) *

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	NO.	DATE	F	REVISION			BY	ı
-			STATE DEPARTMENT (RUCTURES		ORTAT			
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$\frac{1}{1}$	CON SPE		1996	DRAWN BY		PLANS CK'D.		ľ
	5	4" P	RESTRE	SSED	SHE	ΞT		7 11 (
	(GIRD	ER DET	AILS				ı

DRAPED PATTERN

DETAILS' SHT.

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PART LONGIT. SECTION

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STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

NST. 1996 DRAWN BY CMPT PLANS CK'D.

SUPERSTRUCTURE SHEET DETAILS

FILE= G5

GIRDER NOTES

-END OF

GIRDER

2" X 1" BEVEL

3'-2¹/₂" (A)

(EXCEPT WHERE BEVELED ANCHOR PLATE IS USED).

-#4 BAR, EPOXY COATED. PLACE @ STIRRUP SPACING. EMBED INTO GIRDER 1'-3".

· ¾" X ¾" BEVEL

-#4,2'-3" LONG. PLACE AT STIRRUPS BETWEEN ENDS OF GIRDER EXCEPT AT "DETAIL A"

-#4 @ 5" FOR 15'-0" EACH END, #4 @ 1'-0" BETWEEN, 3'-9" LONG TOP OF GIRDER TO BE ROUGH FLOATED AND BROOMED TRANSVERSELY FOR BONDING TO THE SLAB, EXCEPT THE OUTSIDE 15" OF GIRDER, WHICH SHALL RECEIVE A SMOOTH FINISH. AN APPROVED LIQUID BOND BREAKER SHALL BE APPLIED TO THE TOP SURFACE OF THE GIRDER EXCEPT FOR THE CENTER 18". APPLY NO MORE THAN 7 DAYS PRIOR TO POURING THE DECK.

NOX-CRETE: SILCOSEAL 2000 F (2 COATS) MASTER BUILDERS: FIRST COAT - PRECO FORM-COTE SECOND COAT - RHEOFINISH 220

THE GIRDERS SHALL BE PROVIDED WITH A SUITABLE LIFTING DEVICE FOR HANDLING AND ERECTING THE GIRDERS.

PRESTRESSING STRANDS SHALL BE 0.6" - 7 WIRE LOW-RELAXATION STRANDS WITH AN ULTIMATE STRENGTH OF 270,000 PSI AND SHALL BE FLUSH WITH THE ENDS OF THE CURDER

BEND EACH END OF #4 STIRRUPS 41/2" AND #7 STIRRUPS 12".

FOR DIAPHRAGM INSERT & CONNECTION DETAILS, SEE "STEEL DIAPHRAGM" SHEET.

ALL GIRDERS SHALL BE CAST FULL LENGTH AS SHOWN.

SPACING SHOWN FOR *4 STIRRUPS IS FOR GRADE 60 REIN-FORCEMENT. IF THE FABRICATOR WANTS TO BUILD A BAR STEEL CAGE BY WELDING LONGITUDINAL REINFORCEMENT TO THE *4 STIRRUPS, 2 OPTIONS ARE AVAILABLE:

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ENDS OF STRANDS SHALL BE PAINTED WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER AT GIRDER ENDS THAT ARE EXPOSED.

SIDE VIEW & TYP. SECTION IN SPAN

NO BEVEL

#4 STIRRUPS

CLEAR 1'-8¾''

> 11³/₄'' 4⁵/₈''

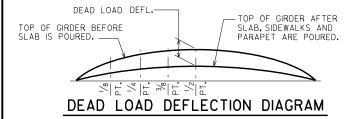
> > 2'-6"

1'-83/4'

- A DETAIL TYP. AT EACH END
- (B) 6 BARS, FULL LENGTH

MIN. LAP

TOP VIEW OF GIRDER ENDS



#4 RAF

3'-2¹/₂" (A)

2 @ 41/2"

41/2"

-BEVELED ANCHOR PLATE, SEE "BEARING DETAILS" SHT.

5 @ 4¹/₂"

* MINIMUM CYLINDER STRENGTH OF CONCRETE © TIME OF TRANSFER OF PRESTRESS FORCE.

						GII	RDE	R DA	TΑ							
	CIDDED	DEAD	LOAD	DEFL	. (IN.)	CONC.		DIA. OF		DRAF	PED P	ATTERN			UNDRAPED	PATTERN
SPAN	GIRDER LENGTH "L"	1/8	1/4	3/8	1/2	STRGTH. f'c (P.S.I.)	"P"	STRAND	TOTAL NO.OF STRANDS	f'ci (P.S.I.) X	"A"	(IN. "B" MIN.	"B" MAX.	"C"	TOTAL NO.OF STRANDS	f'ci (P.S.I.) X

#4 STIRRUPS

-#3 BARS EPOXY COATED

#3 BARS @ 6"

21'-0"

NO. DATE REVISION BY

STATE OF WISCONSIN STRUCTURES DESIGN SECTION

STRUCTURES DESIGN SECTION

STRUCTURE

CONST. 1996 DRAWN BY

PLANS CKD.

SHEET

GIRDER DETAILS

= G54W,DGN

8

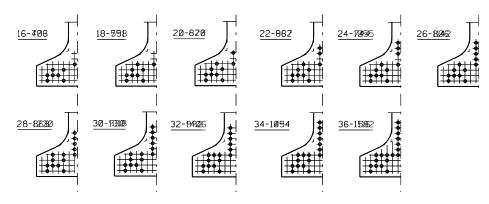
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END OF

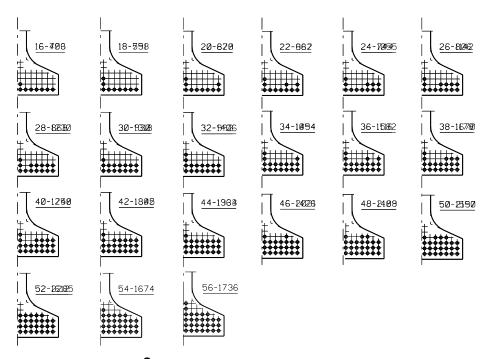
#7 STIRRUPS IN PAIRS. —

11/2" DIA. HOLE AT ABUT, END

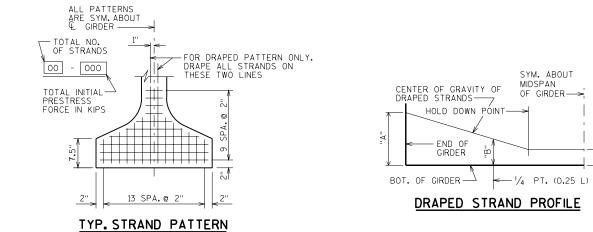
GIRDER

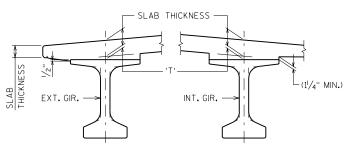


STANDARD ARRANGEMENTS TO RAISE CENTER OF GRAVITY TO AVOID DRAPING OF STRANDS



ARRANGEMENT AT & SPAN - FOR GIRDERS WITH DRAPED STRANDS





SLAB HAUNCH DETAIL

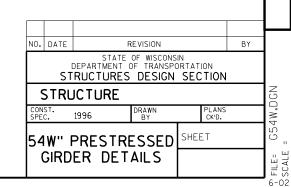
IF $1\frac{1}{4}$ " Minimum haunch height at edge of girder cannot be maintained, the grade line may be revised by the engineer at the option of the contractor. If grade line is raised from plan profile, contact the structures section. Plan slab thickness shall be held.

TO DETERMINE 'T', ELEV. OF TOP OF GIR'S, AT $\widehat{\mathbb{Q}}$ OF SUBSTRUCTURE UNITS & AT 1/8 Points of each span shall be taken. Then follow this PROCESS:

- TOP OF DECK ELEV. AT FINAL GRADE
 TOP OF GIRDER ELEVATION
 + DEAD LOAD DEFLECTION
 SLAB THICKNESS

- = HAUNCH HEIGHT 'T'

8



STATE PROJECT NUMBER

BILL OF BARS NOTE: THE FIRST OR FIRST TWO DIGITS OF THE BAR MARK SIGNIFIES THE BAR SIZE.

	_		<u> </u>			BAK MARK SIGNIFIES THE BAK SIZE.
BAR MARK	C047	NO. REO'D.	LENGTH	&EN,	BAR SERIES	LOCATION
					-	
						AUSDIOS I SUOTIL AND CHOIL D AND

⚠ LENGTH SHOWN FOR BAR IS AN AVERAGE LENGTH AND SHOULD ONLY BE USED FOR BAR WEIGHT CALCULATIONS. SEE BAR SERIES TABLE FOR ACTUAL LENGTHS.

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PART LONGIT. SECTION

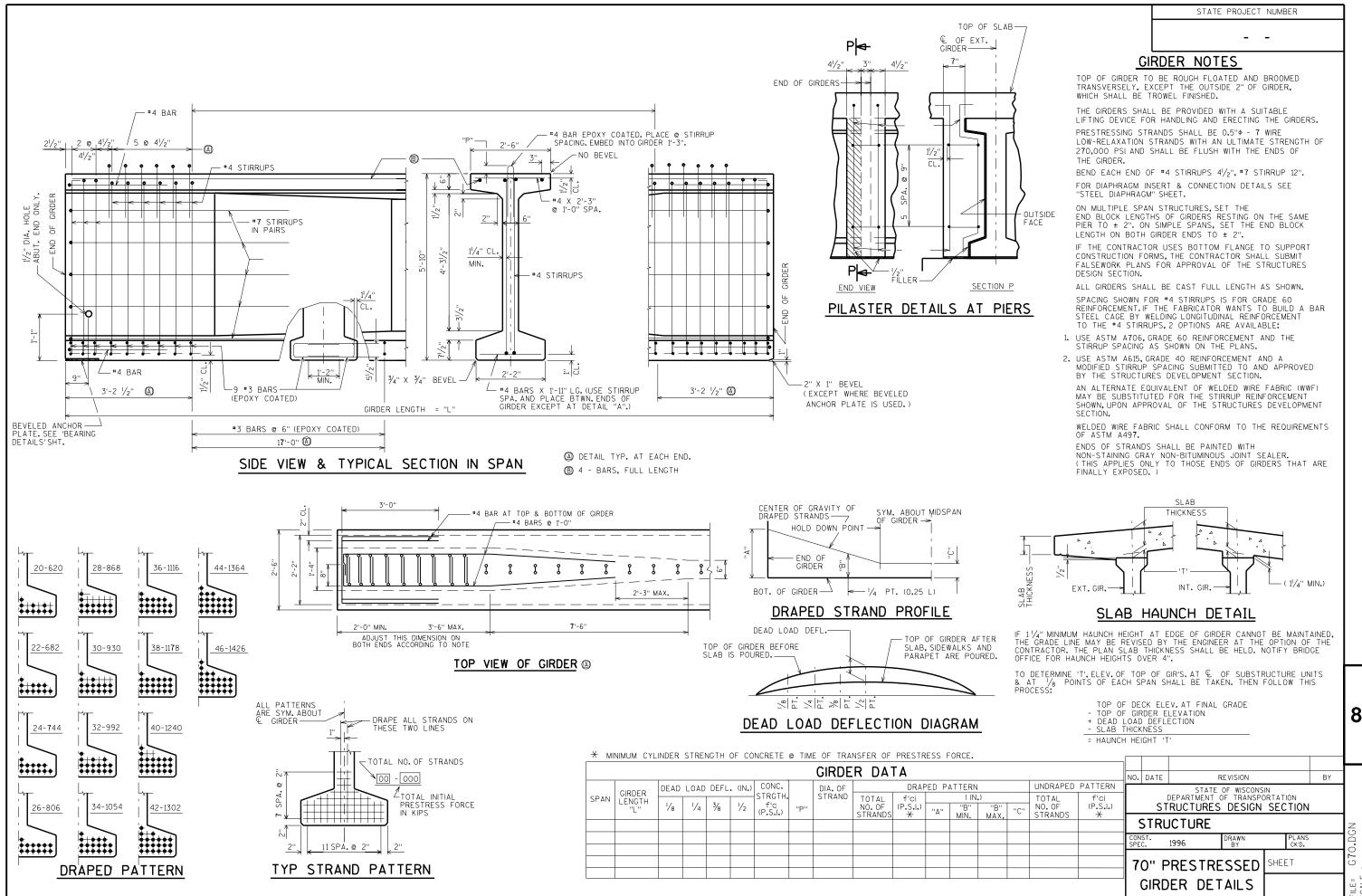
NO. DATE

STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

SUPERSTRUCTURE

DETAILS



∄ 5 7-01 STATE PROJECT NUMBER

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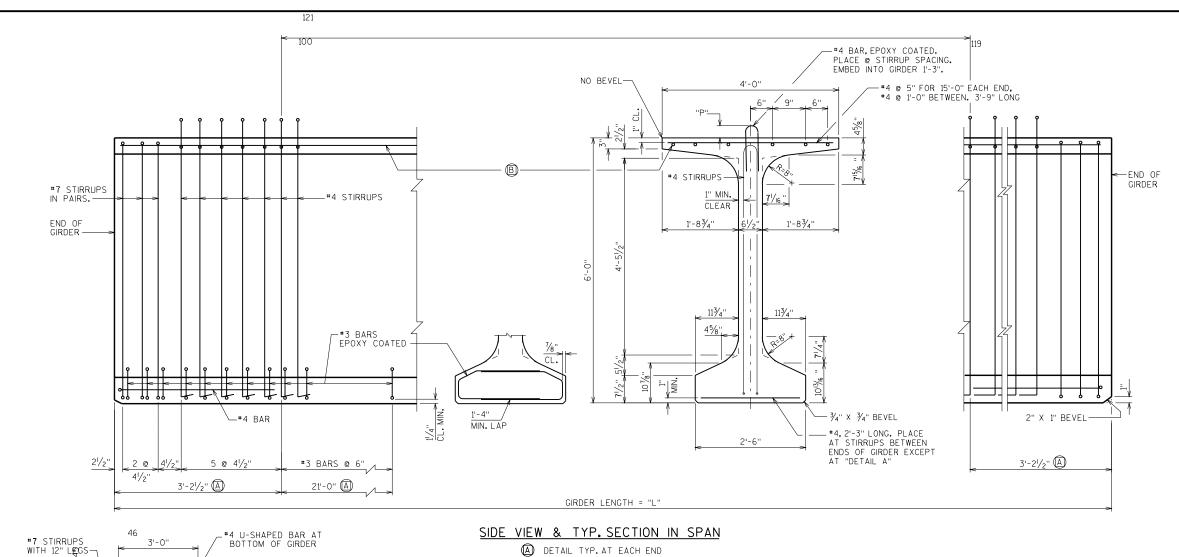
STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

CONST. 1996 DRAWN
BY

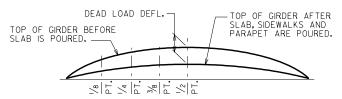
CONST. 1996 CKD.

SUPERSTRUCTURE
DETAILS



- (A) DETAIL TYP. AT EACH END
- B 6 BARS, FULL LENGTH

TOP VIEW OF GIRDER ENDS



DEAD LOAD DEFLECTION DIAGRAM

* MINIMUM CYLINDER STRENGTH OF CONCRETE AT TIME OF TRANSFER OF PRESTRESS FORCE.

							(GIRDEF	RDAT	Α						
	GIRDER	DEAD	LOAD	DEFL	. (IN.)			DIA. OF		DRAF	ED P	ATTERN			UNDRAPED	
SPAN	LENGTH "L"	1/8	1/4	3/8	1/2	STRGTH. F'C (P.S.I.)	"P"	STRAND	TOTAL NO. OF STRANDS	f'ci (P.S.I.) X	"A"	(IN. "B" MIN.	.) "B" MAX.	"C"	TOTAL NO.OF STRANDS	f'ci (P.S.I.) X
3	4	5	6	7	8	9	10	24	11	12	13	14	15	16	17	18
19																

GIRDER NOTES

TOP OF GIRDER TO BE ROUGH FLOATED AND BROOMED TRANSVERSELY FOR BONDING TO THE SLAB, EXCEPT THE OUTSIDE 15" OF GIRDER, WHICH SHALL RECEIVE A SMOOTH FINISH, AN APPROVED LIQUID BOND BREAKER SHALL BE APPLIED TO THE TOP SURFACE OF THE GIRDER EXCEPT FOR THE CENTER 18". APPLY NO MORE THAN 7 DAYS PRIOR TO POURING THE DECK.

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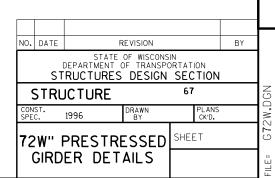
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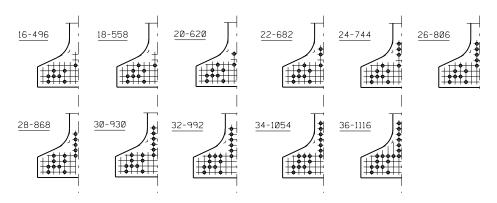
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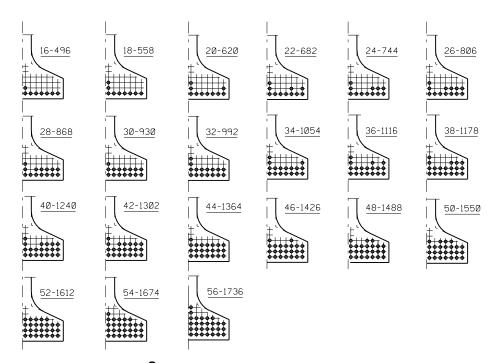
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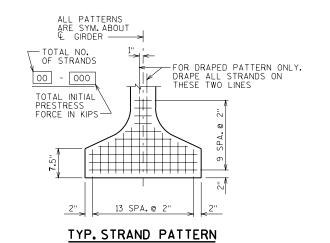
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STANDARD ARRANGEMENTS TO RAISE CENTER OF GRAVITY TO AVOID DRAPING OF STRANDS

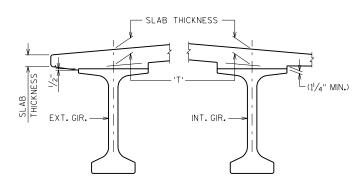


ARRANGEMENT AT & SPAN - FOR GIRDERS WITH DRAPED STRANDS



-CENTER OF GRAVITY OF DRAPED STRANDS END OF GIRDER : --- HOLD DOWN POINT - SYM. ABOUT MIDSPAN OF GIRDER - 1/4 POINT (0.25 L) BOTTOM OF GIRDER

DRAPED STRAND PROFILE



SLAB HAUNCH DETAIL

IF $1\frac{1}{4}$ " Minimum haunch height at edge of girder cannot be maintained, the grade line may be revised by the engineer at the option of the contractor. If grade line is raised from plan profile, contact the structures section. Plan slab thickness shall be held.

TO DETERMINE 'T', ELEV. OF TOP OF GIR'S. AT $\hat{\mathbb{Q}}$ OF SUBSTRUCTURE UNITS & AT 1/8 Points of each span shall be taken. Then follow this process:

- TOP OF DECK ELEV. AT FINAL GRADE
 TOP OF GIRDER ELEVATION
 + DEAD LOAD DEFLECTION
 SLAB THICKNESS

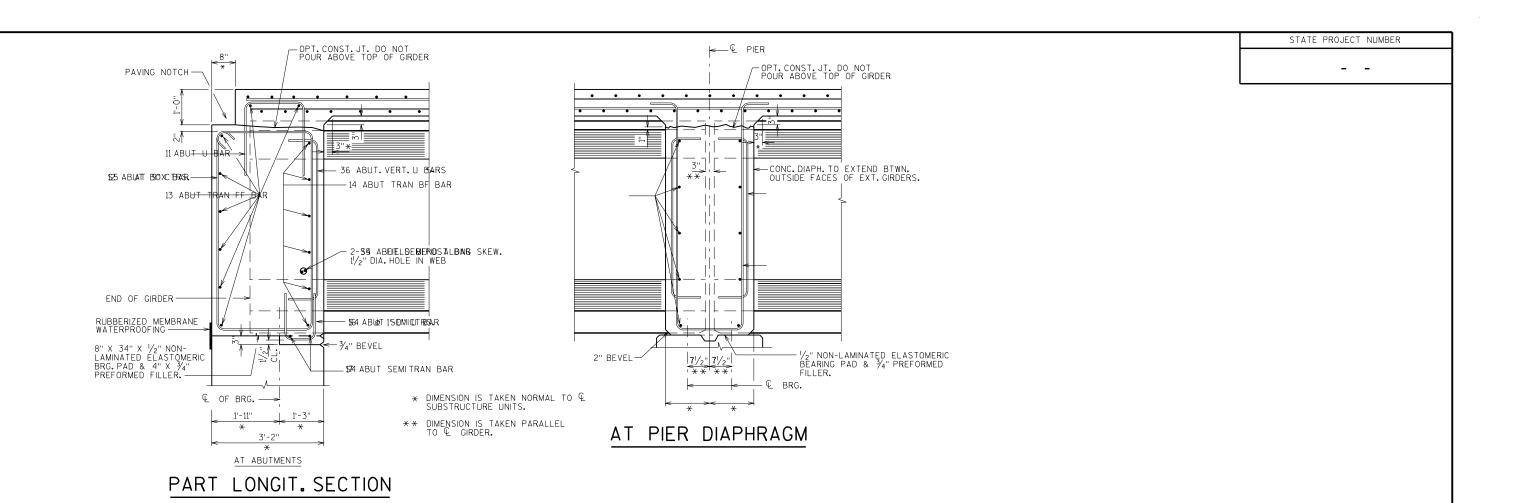
- = HAUNCH HEIGHT 'T'

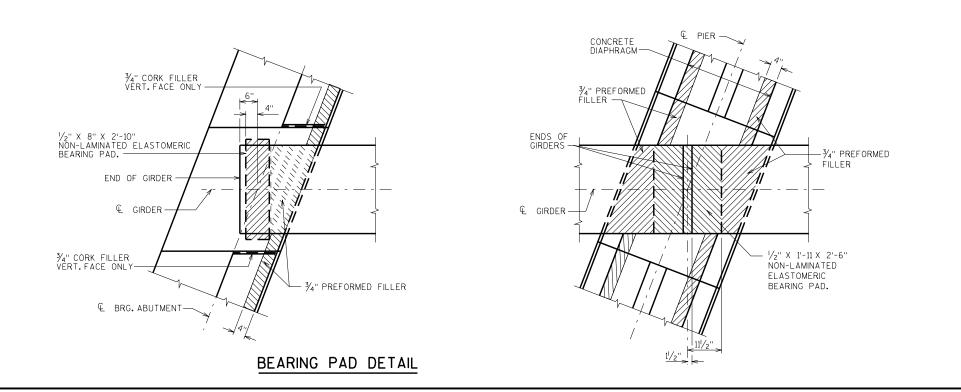
NO. DATE REVISION BY STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION 69 STRUCTURE PLANS CK'D. SHEET

72W" PRESTRESSED GIRDER DETAILS

70 11-02

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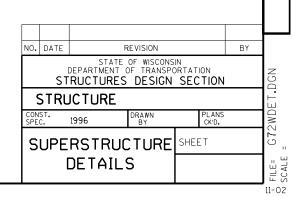
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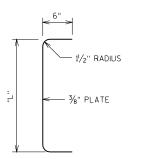
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TABLE

	_		_	
GIRDER HEIGHT	DIM. "A"	DIM. "B"	DIM. "L"	* DIM. * "X"
28"	1'-07/8''	5 1/8"	91/2"	21/4"
36"	1'-27/8''	97/8"	1'-11/2"	31/4"
45"	1'-53/8''	1'-17/8''	1'-51/2"	21/4"
54"	1'-77/8"	1'-57/8''	1'-91/2"	41/4"
54W"	1'-91/8"	1'-5 1/8''	1'-91/2"	41/4"

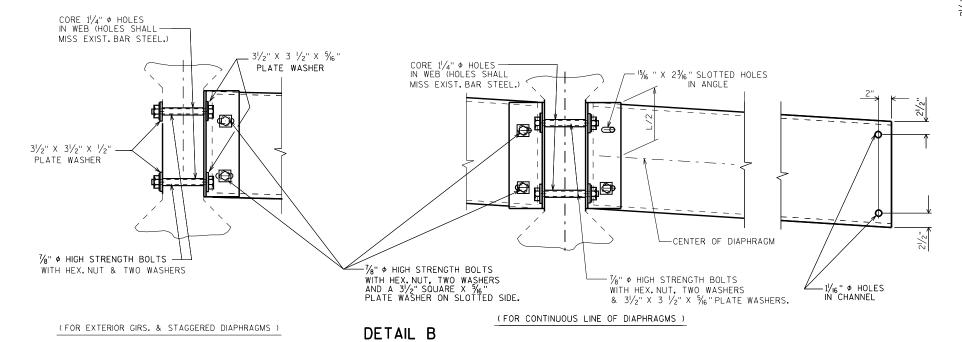


SECTION THRU ALTERNATE DIAPHRAGM

*DIM "X" = 21/2" FOR ALTERNATE PLATE DIAPHRAGM

TOP OF DECK C 10 X 15.3 FOR 28" BEAMS C 12 X 20.7 FOR 36" BEAMS MC 18 X 42.7 FOR 45", 54" & 54W" BEAMS OR ALTERNATE MADE FORM 3/8" PLATE - SEE DETAIL B EXTERIOR GIRDER INTERIOR GIRDER

PART TRANSVERSE SECTION AT DIAPHRAGM



NOTES

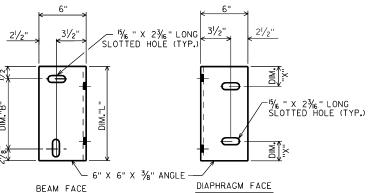
ALL DIAPHRAGM MATERIAL AND CORED HOLES SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "STEEL DIAPHRAGM", STRUCTURE, EACH.

EACH DIAPHRAGM BETWEEN GIRDERS SHALL CONSTITUTE ONE UNIT.

STATE PROJECT NUMBER

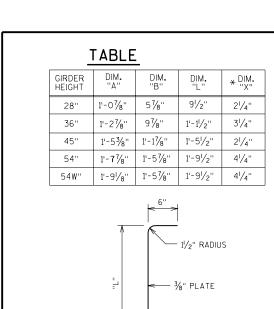
ALL DIAPHRAGM STRUCTURAL STEEL SHALL BE ASTM A709 GRADE 36. ALL BOLTS, NUTS AND WASHERS SHALL BE ASTM A325 TYPE 1.

ALL DIAPHRAGM STRUCTURAL STEEL SHOWN SHALL BE HOT-DIPPED GALVANIZED. ALL BOLTS, NUTS AND WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C. GALVANIZED NUTS SHALL BE TAPPED OVERSIZE IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A563 AND SHALL MEET THE REQUIREMENTS OF SUPPLEMENTARY REQUIREMENT S1 OF ASTM A563, LUBRICANT AND TEST FOR COATED



DIAPHRAGM SUPPORT

NO. DATE BY STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION STRUCTURE 1996 SHEET STEEL DIAPHRAGM



TOP OF DECK C 10 X 15.3 FOR 28" BEAMS C 12 X 20.7 FOR 36" BEAMS MC 18 X 42.7 FOR 45", 54" & 54W" BEAMS OR ALTERNATE MADE FROM 3/8" PLATE - SEE DETAIL B EXTERIOR GIRDER INTERIOR GIRDER

NOTES

ALL DIAPHRAGM MATERIAL NOT EMBEDDED IN THE CONCRETE GIRDER SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "STEEL DIAPHRAGM", STRUCTURE, EACH.

EACH DIAPHRAGM BETWEEN GIRDERS SHALL CONSTITUTE ONE UNIT.

STATE PROJECT NUMBER

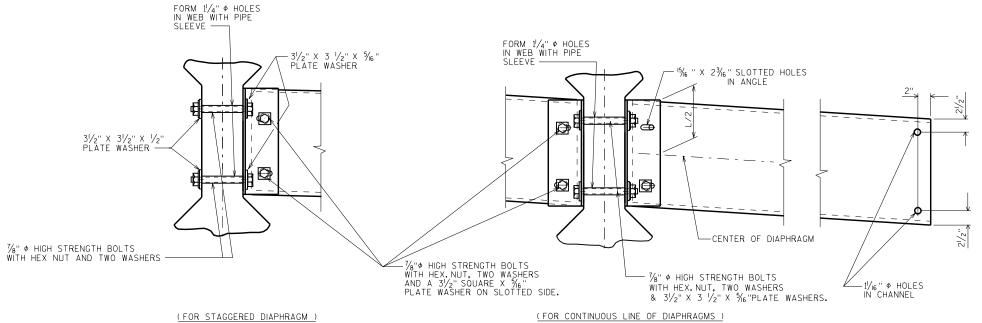
ALL DIAPHRAGM STRUCTURAL STEEL SHALL BE ASTM A709 GRADE 36. ALL BOLTS, NUTS AND WASHERS SHALL BE ASTM A325 TYPE 1.

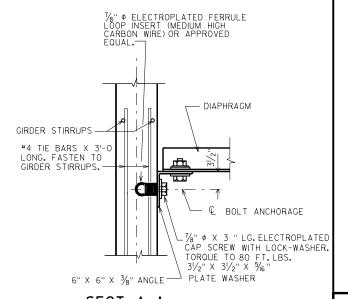
ALL DIAPHRAGM STRUCTURAL STEEL SHOWN SHALL BE HOT-DIPPED GALVANIZED. ALL BOLTS, NUTS AND WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C. GALVANIZED NUTS SHALL BE TAPPED OVERSIZE IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A563 AND SHALL MEET THE REQUIREMENTS OF SUPPLEMENTARY REQUIREMENT S1 OF ASTM A563, LUBRICANT AND TEST FOR COATED

SECTION THRU ALTERNATE DIAPHRAGM

*DIM "X" = 21/2" FOR ALTERNATE PLATE DIAPHRAGM

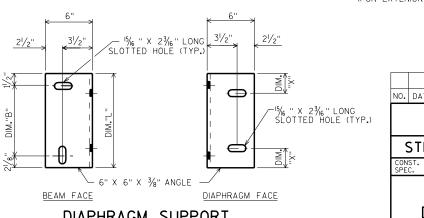
PART TRANSVERSE SECTION AT DIAPHRAGM





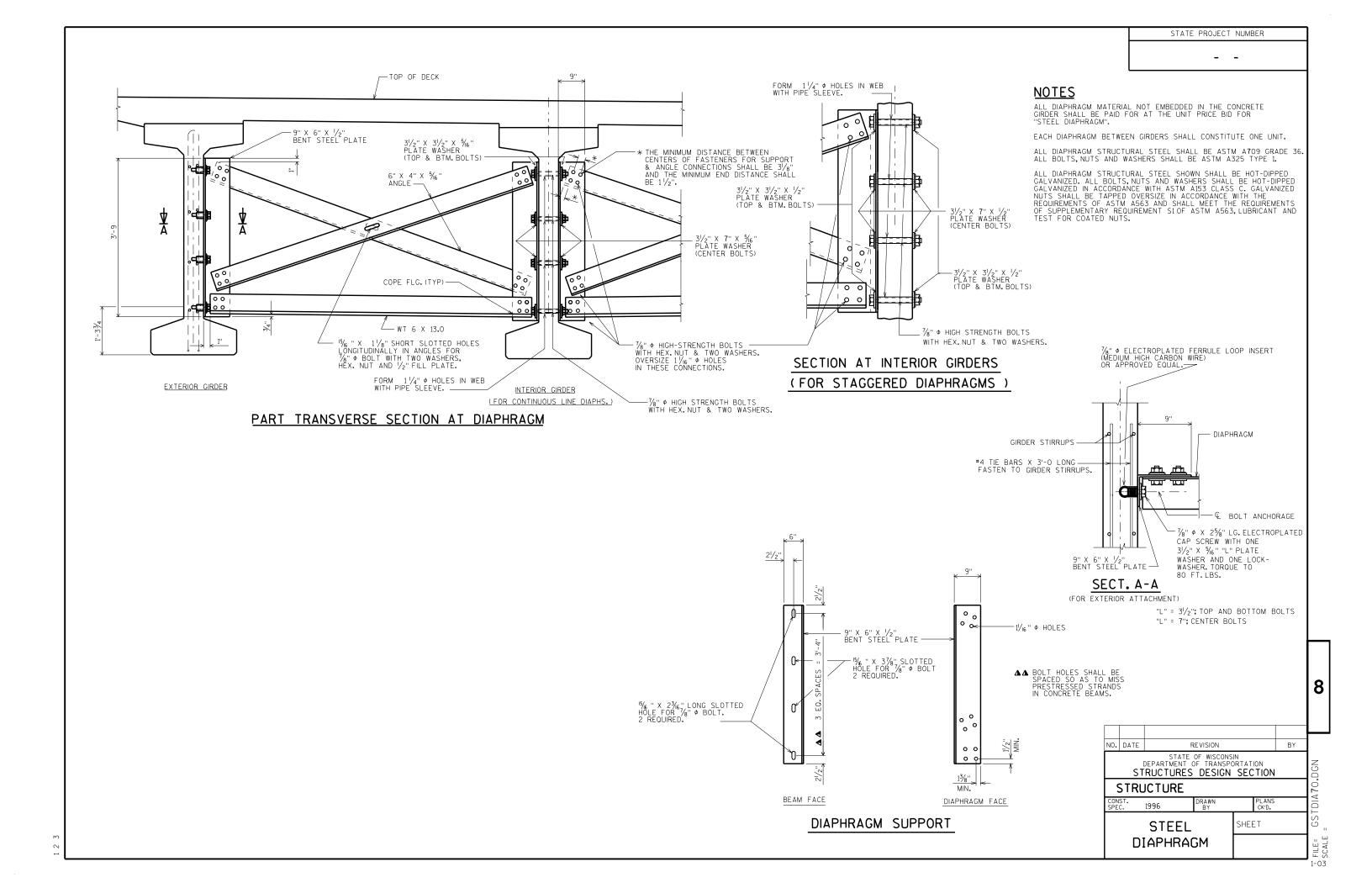
DETAIL B

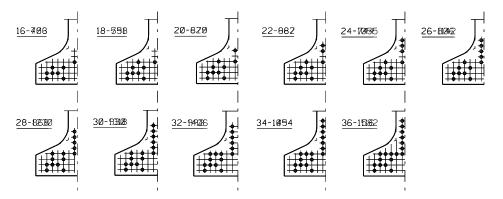
SECT. A-A (FOR EXTERIOR ATTACHMENT)



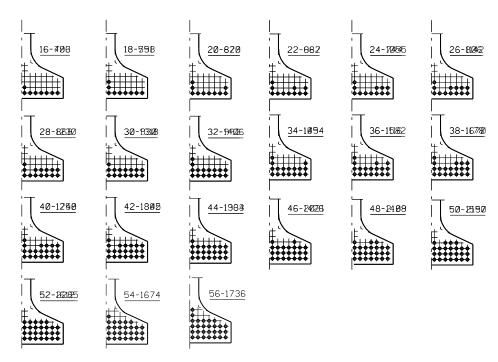
DIAPHRAGM SUPPORT

NO. DATE BY STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION STRUCTURE 1996 SHEET STEEL DIAPHRAGM

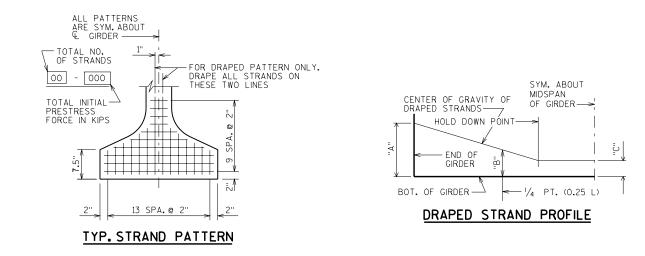


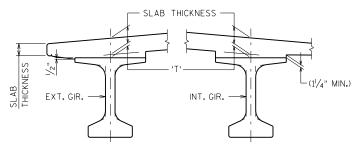


STANDARD ARRANGEMENTS TO RAISE CENTER OF GRAVITY TO AVOID DRAPING OF STRANDS



ARRANGEMENT AT & SPAN - FOR GIRDERS WITH DRAPED STRANDS





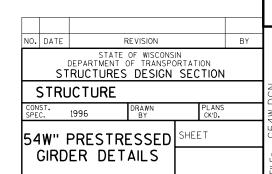
SLAB HAUNCH DETAIL

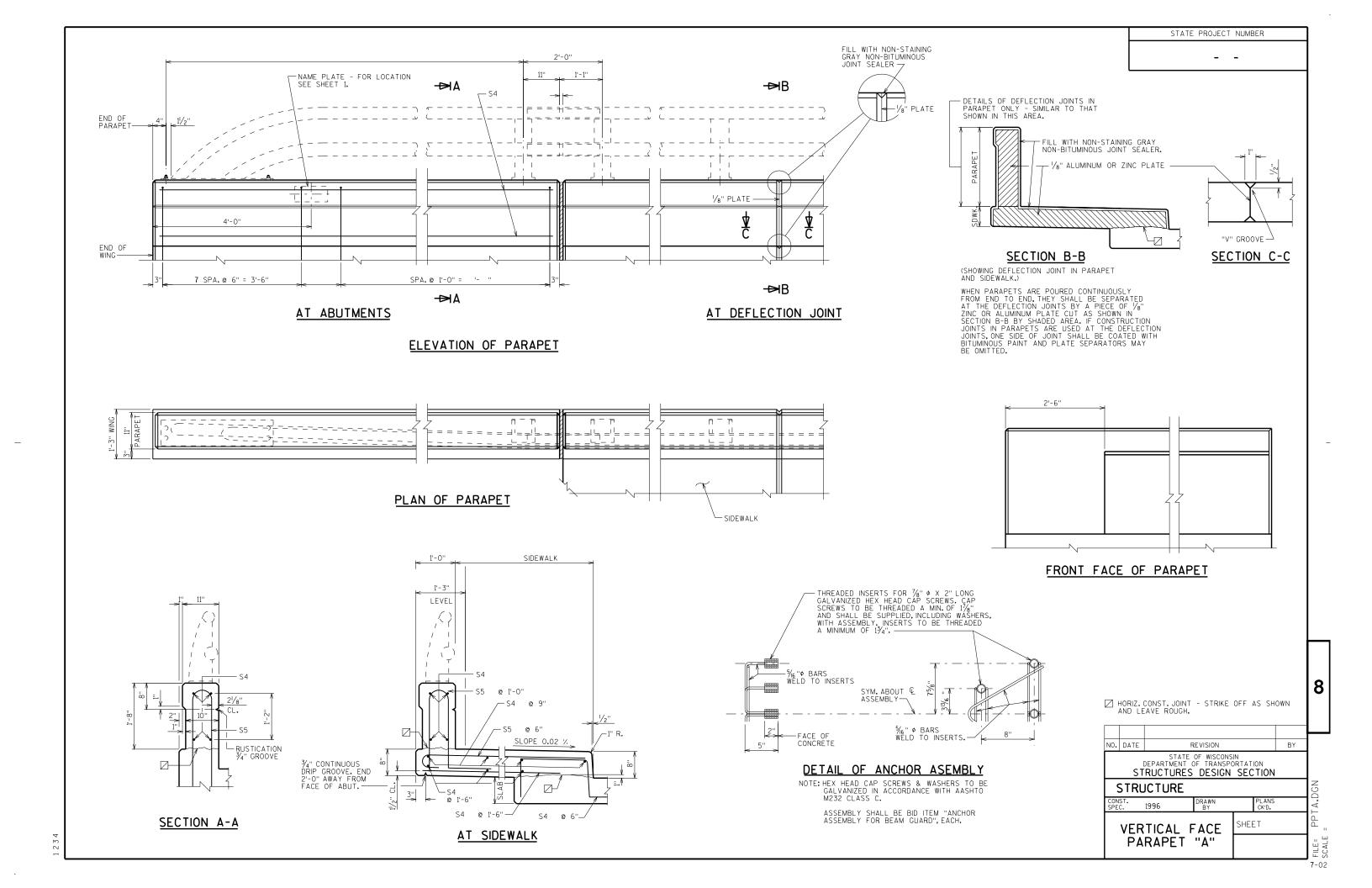
IF $1 \slash\hspace{-0.05cm} I_4$ minimum haunch height at edge of girder cannot be maintained, the grade line may be revised by the engineer at the option of the contractor. If grade line is raised from plan profile, contact the structures section. Plan slab thickness shall be held.

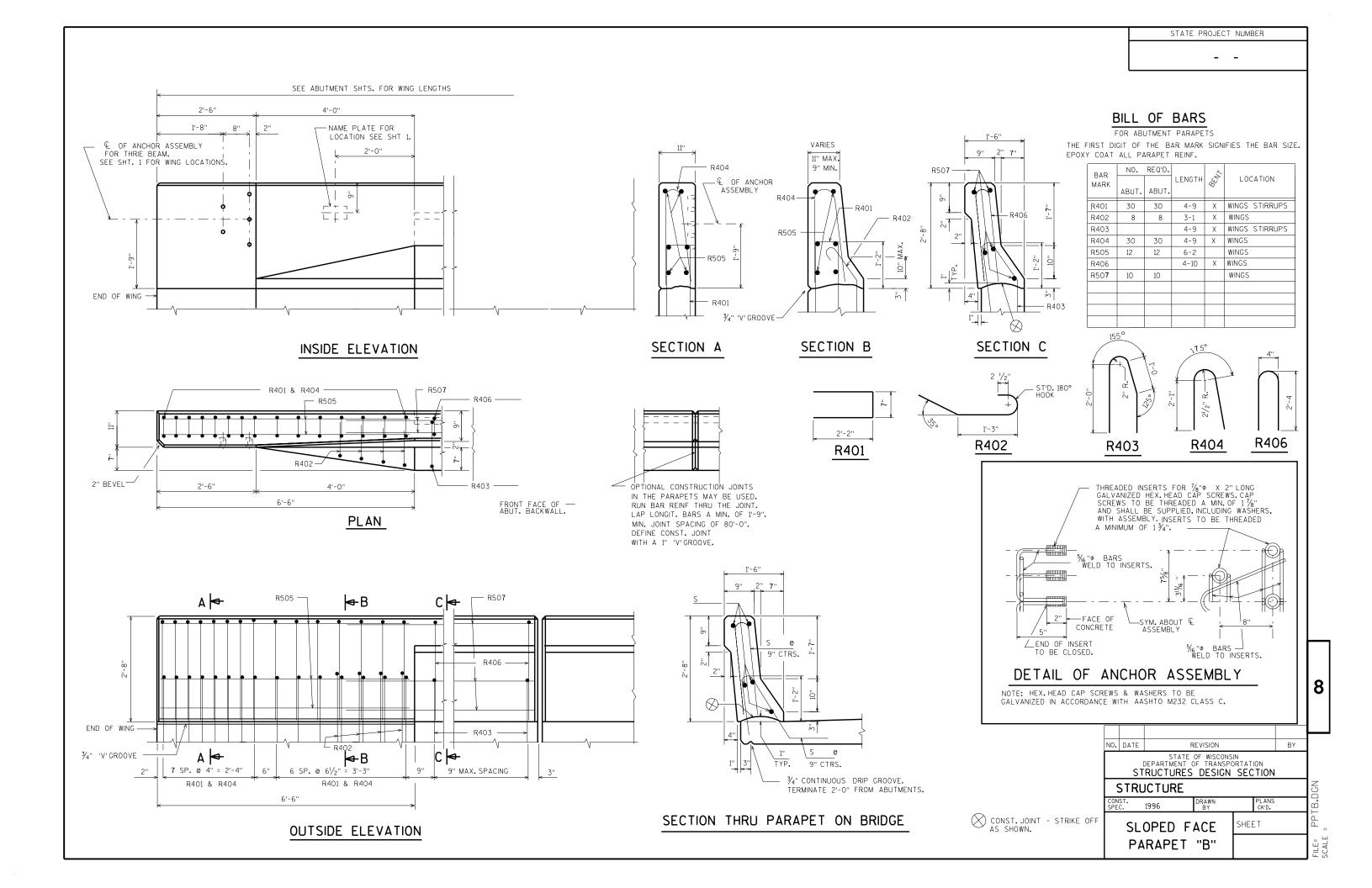
TO DETERMINE 'T', ELEV. OF TOP OF GIR'S. AT $\hat{\mathbb{Q}}_-$ OF SUBSTRUCTURE UNITS & AT $1/_{8}$ POINTS OF EACH SPAN SHALL BE TAKEN. THEN FOLLOW THIS PROCESS:

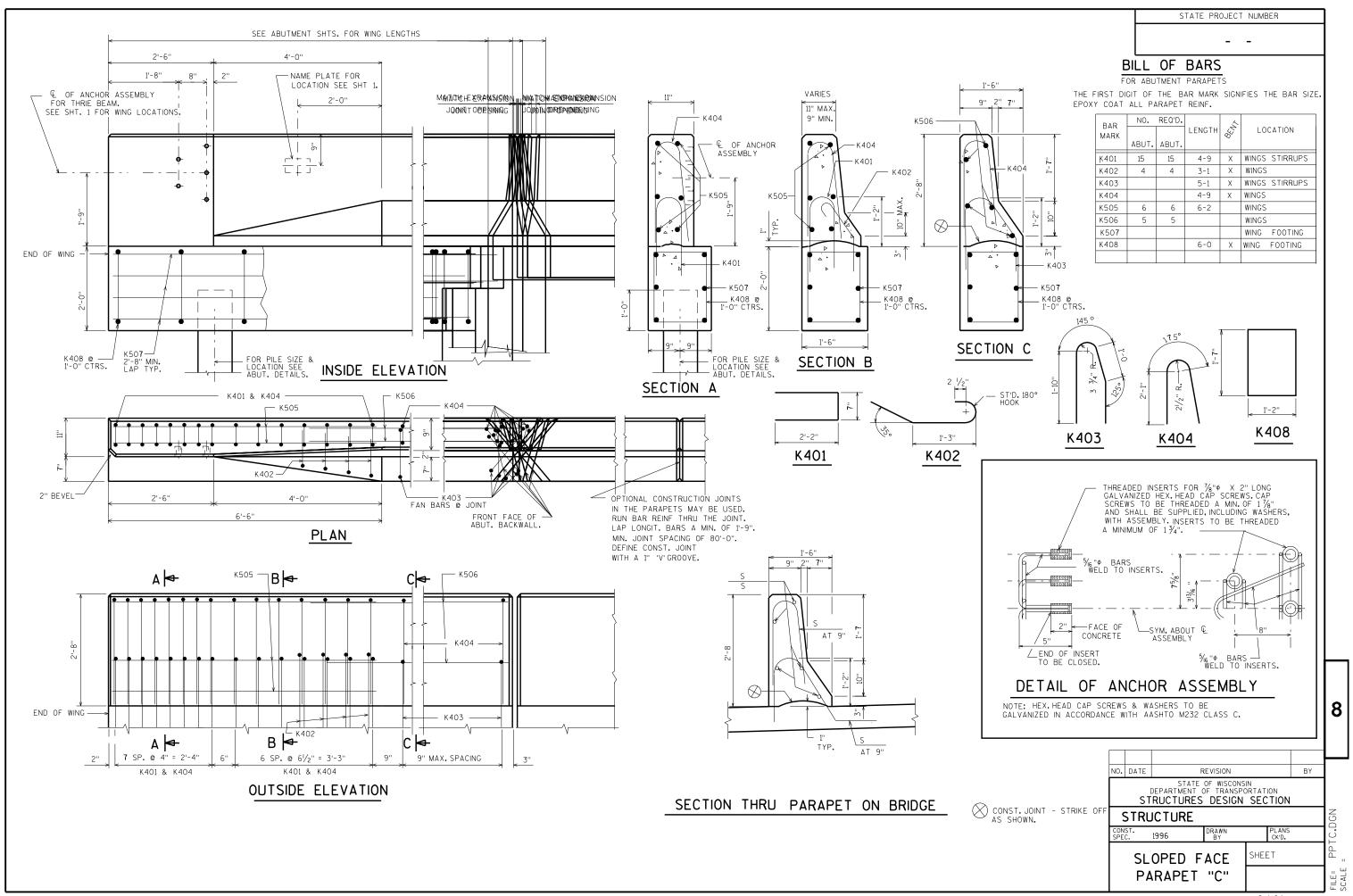
- TOP OF DECK ELEV. AT FINAL GRADE
 TOP OF GIRDER ELEVATION
 + DEAD LOAD DEFLECTION
 SLAB THICKNESS

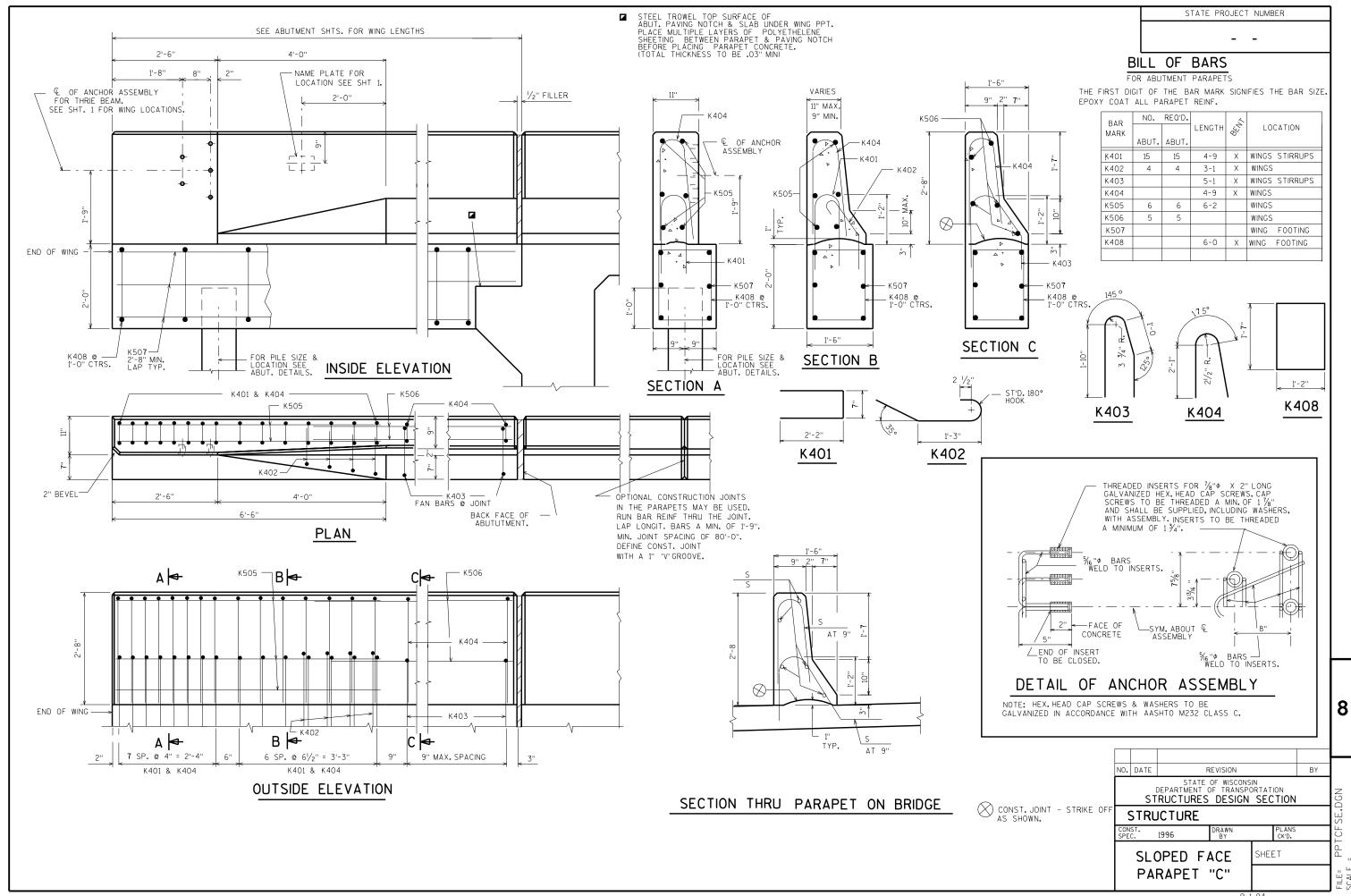
- = HAUNCH HEIGHT 'T'

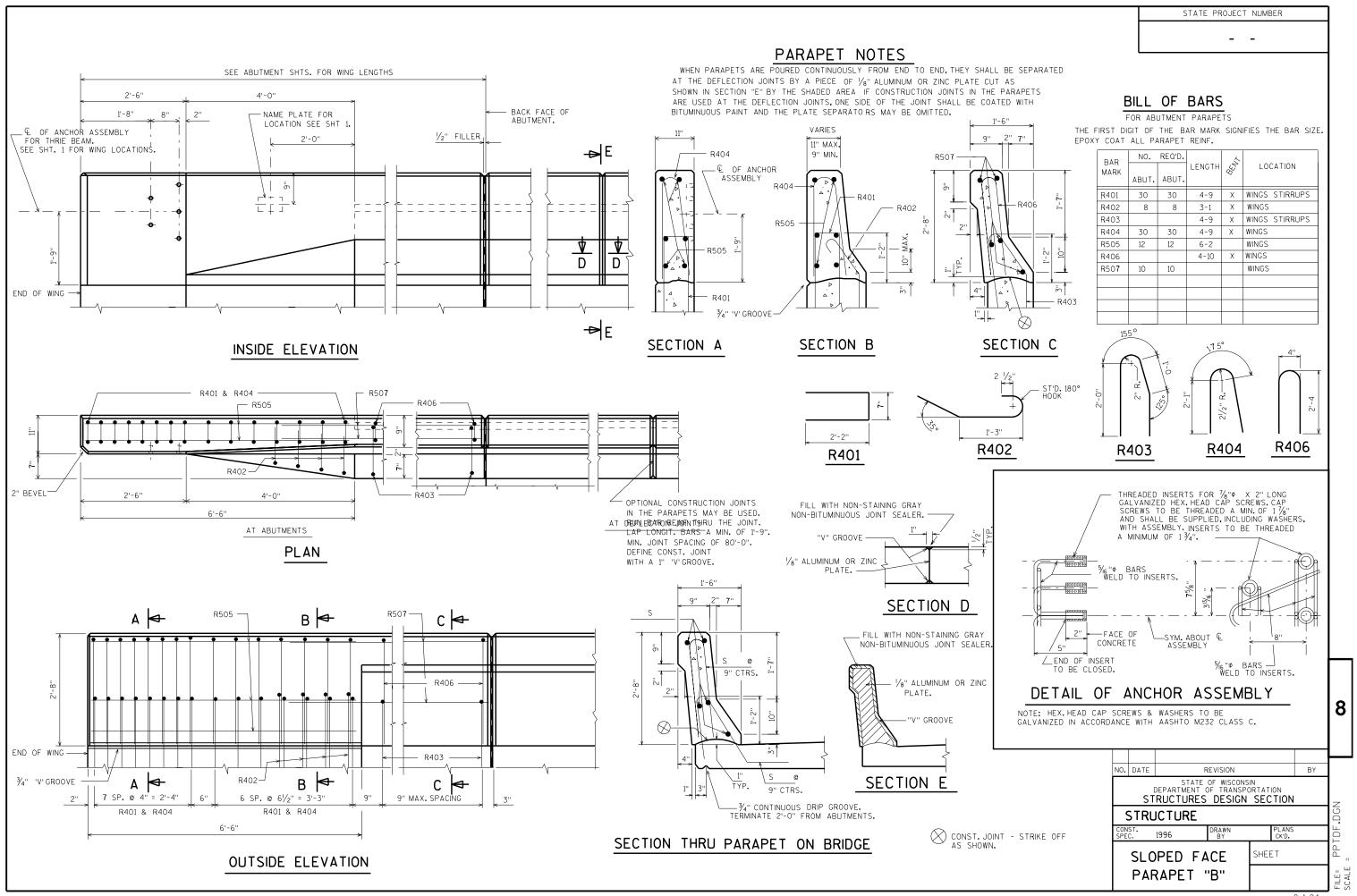


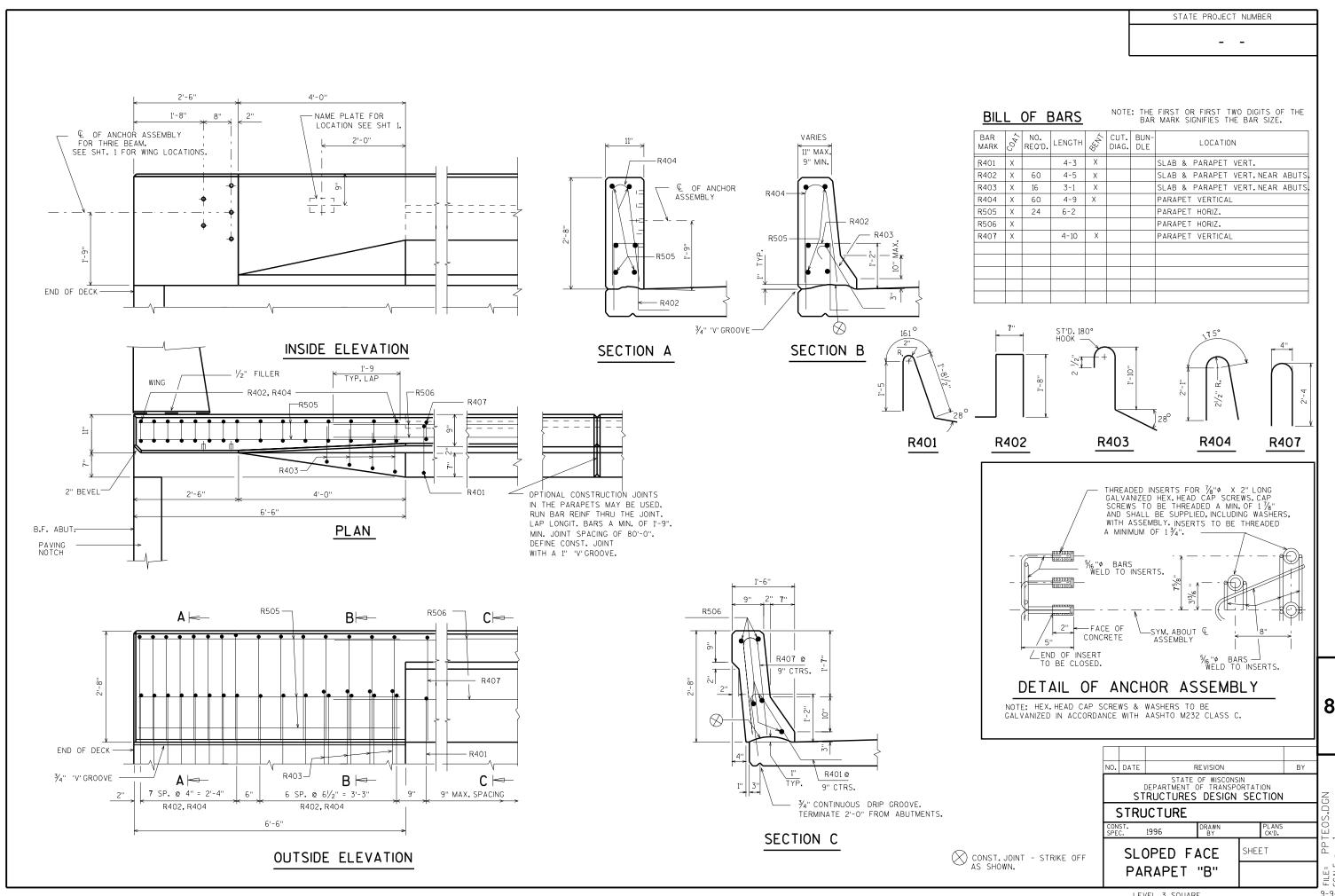




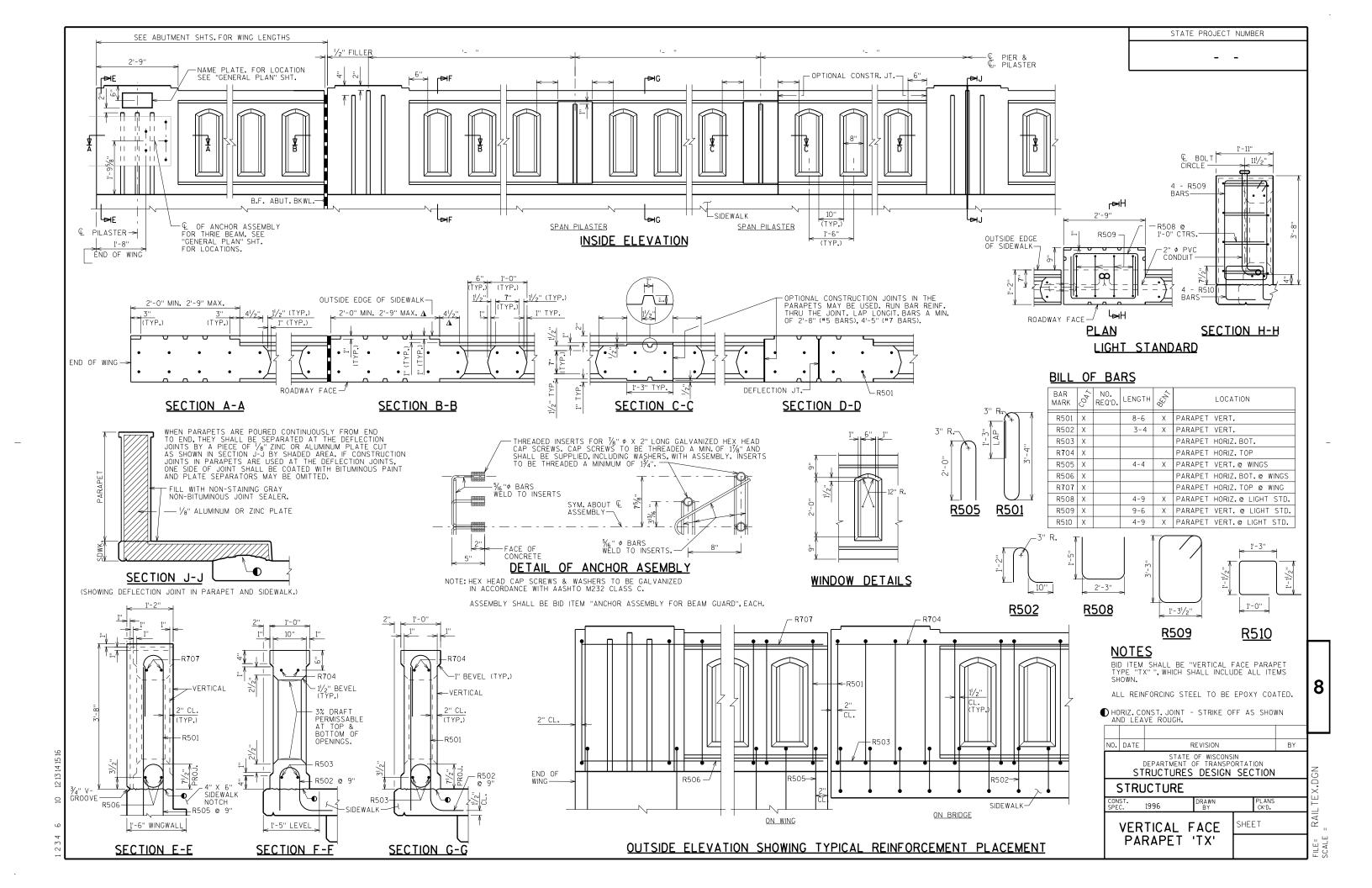


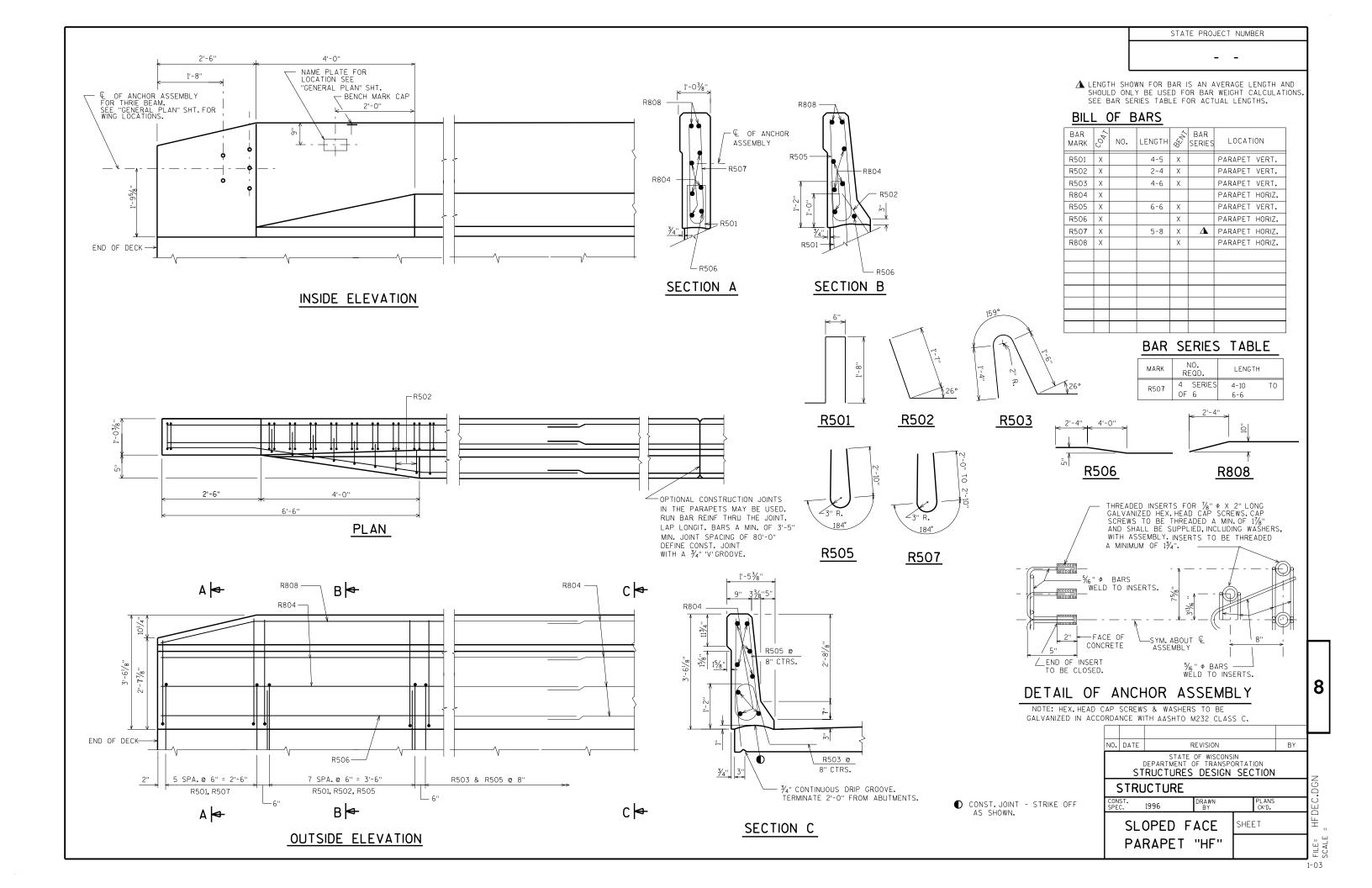


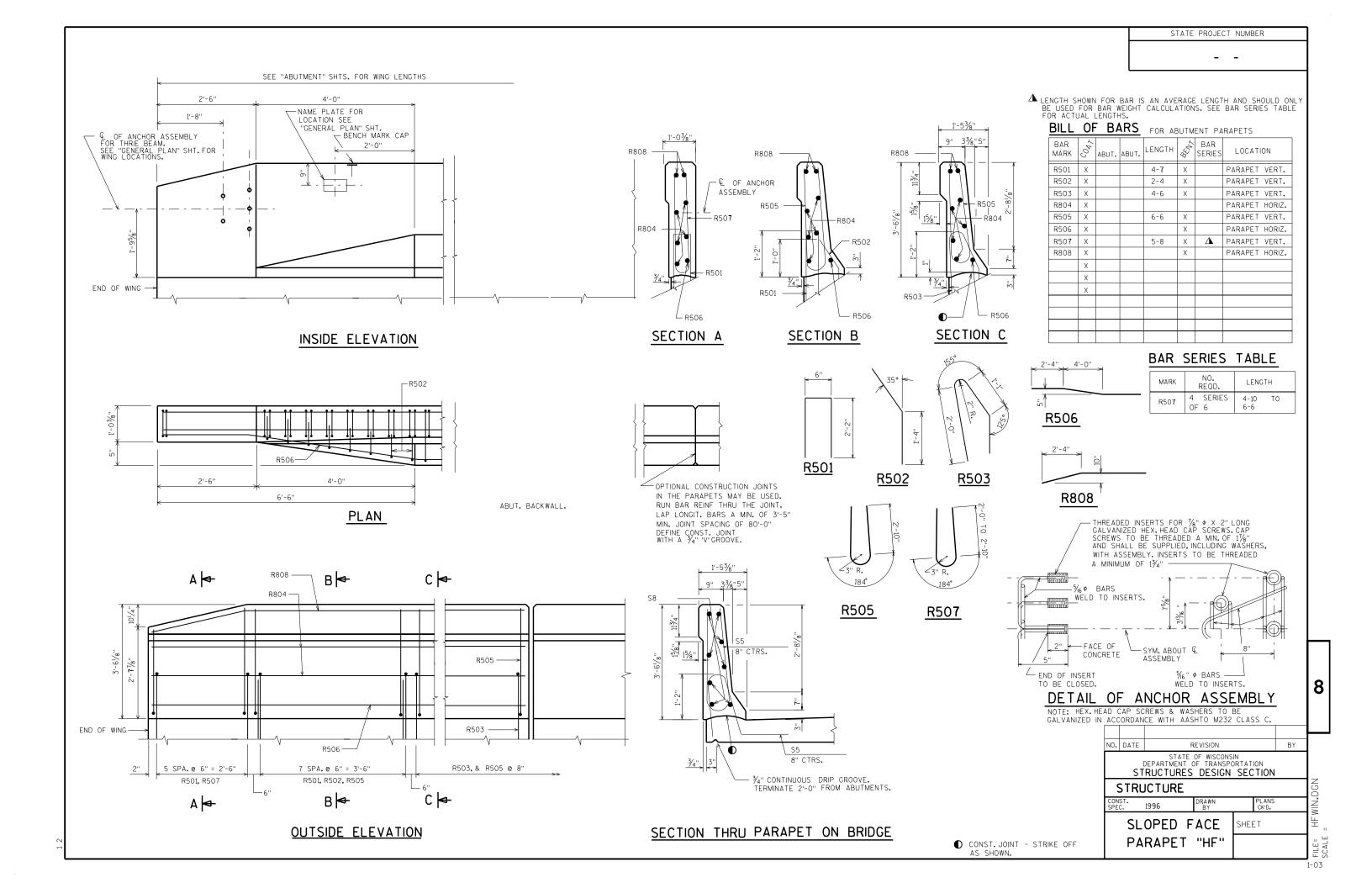


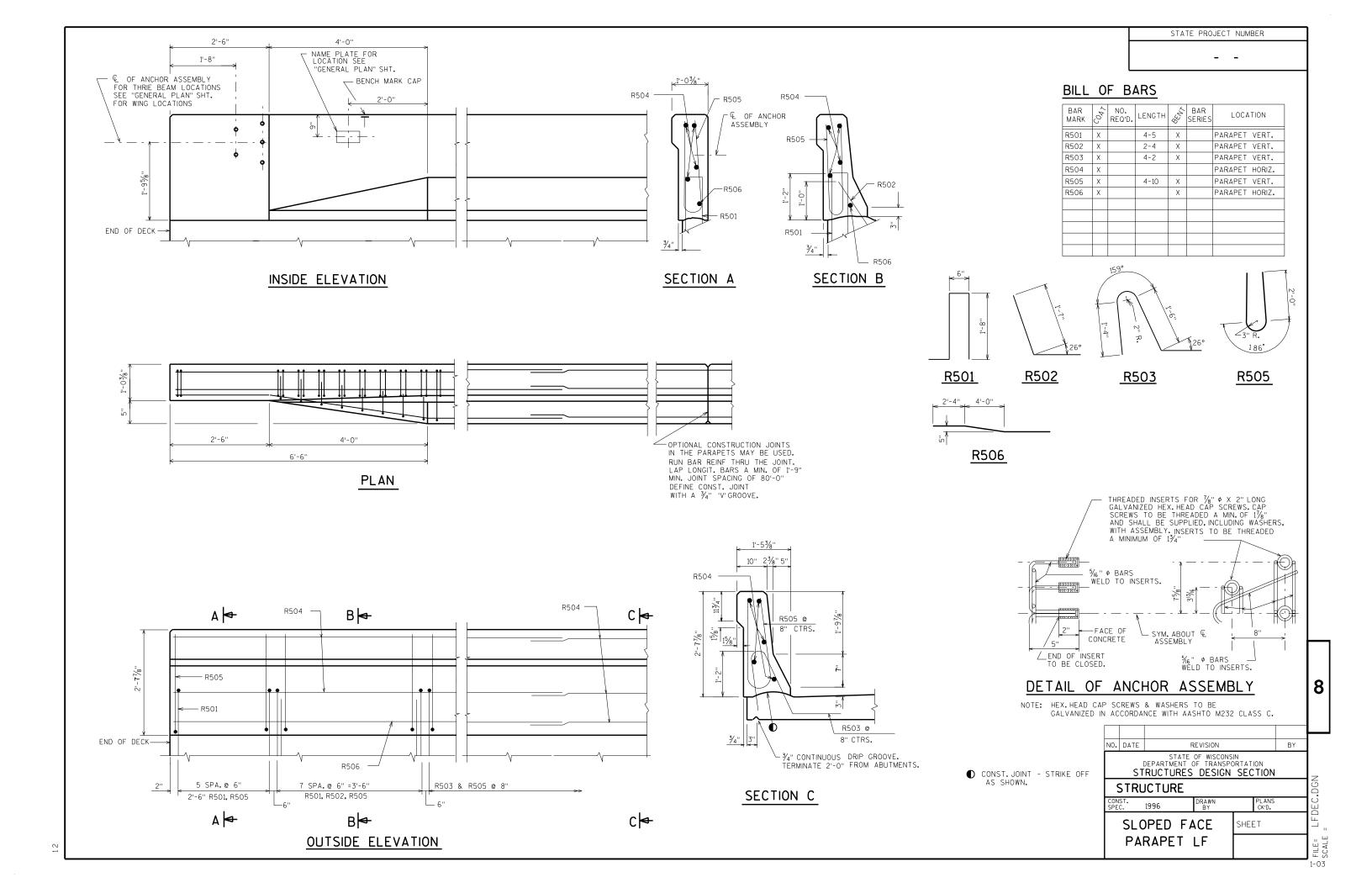


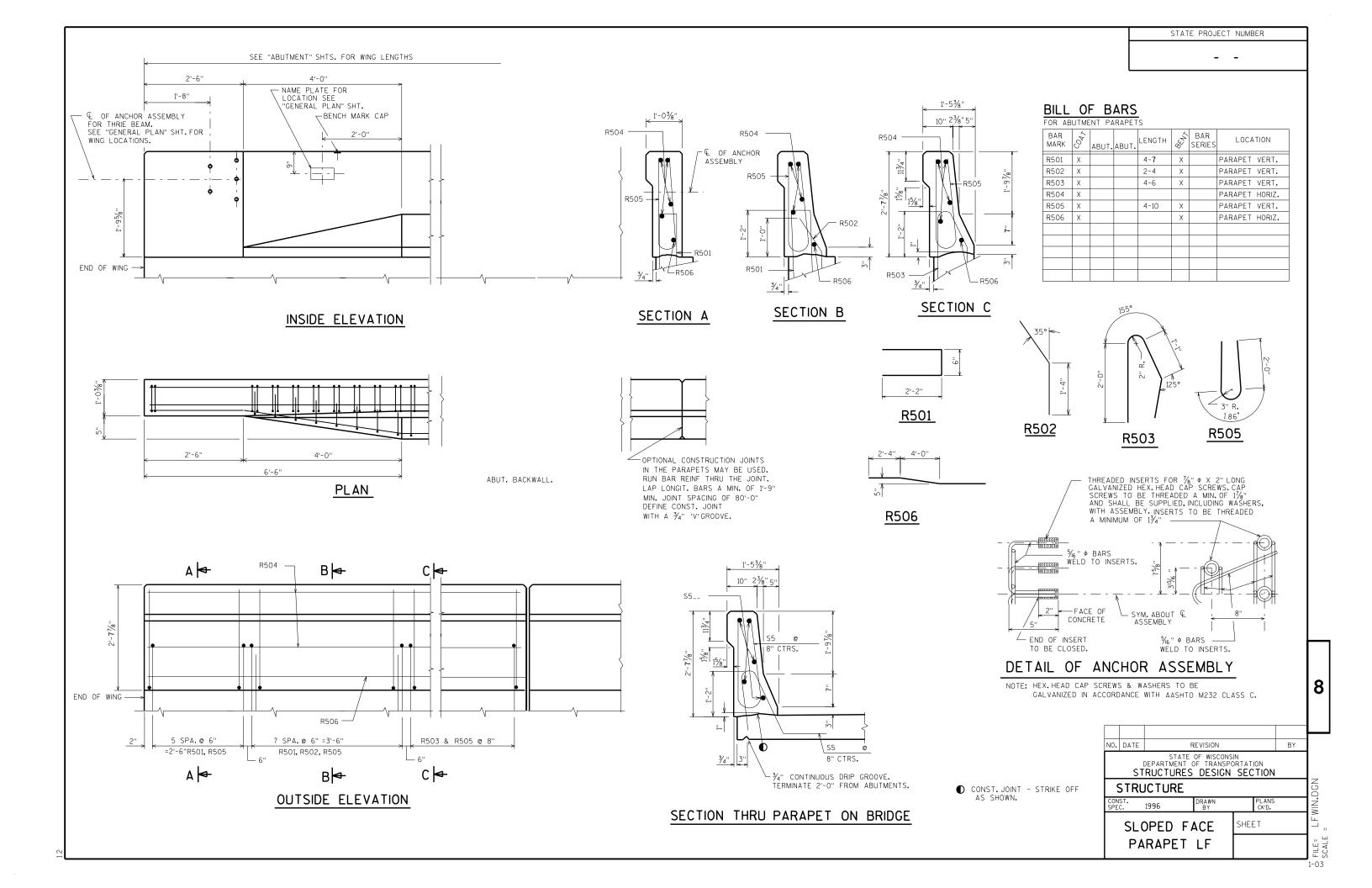
LEVEL 3 SQUARE

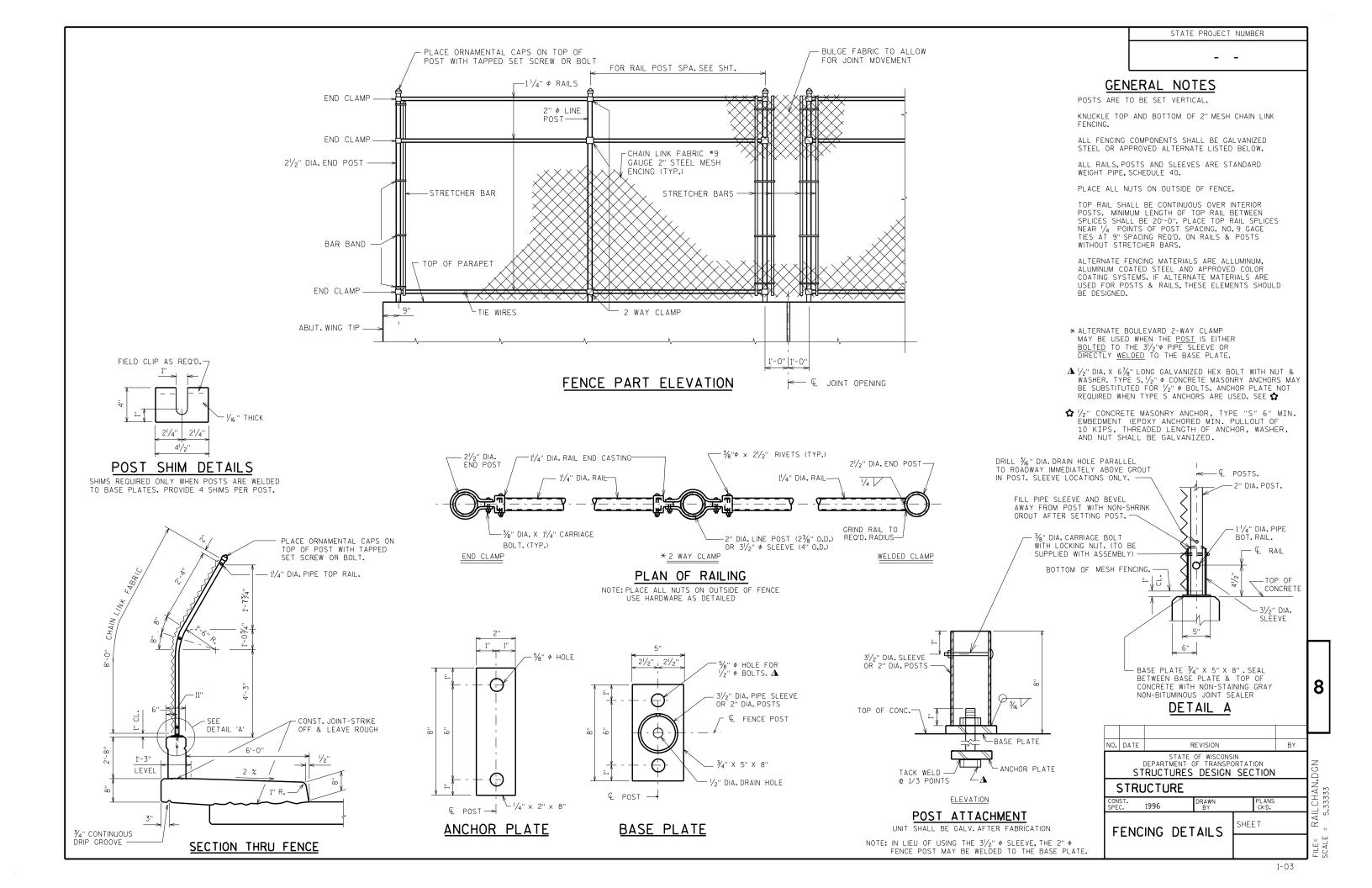


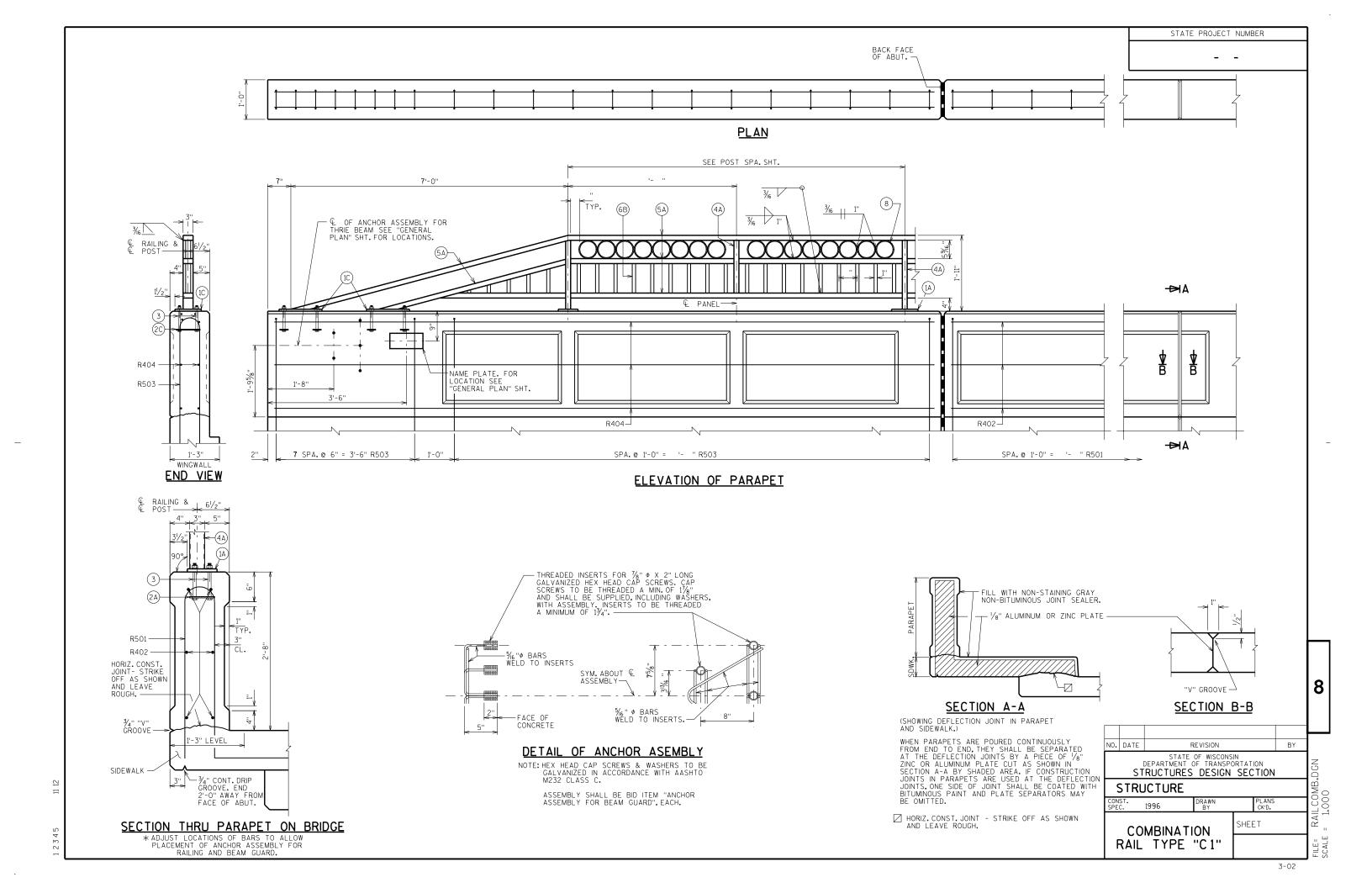


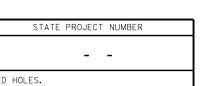


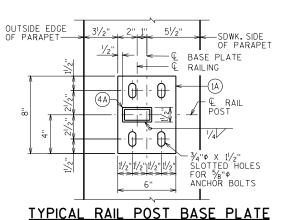












SHIM AS REQ'D. TO ALIGN RAILING. MIN. OF ONE PER POST.

-(4A)

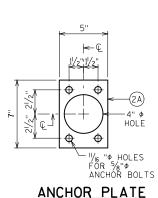
TACK WELD

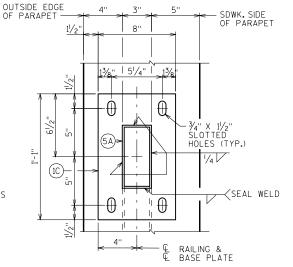
AT 1/3 POINTS

(1A)—

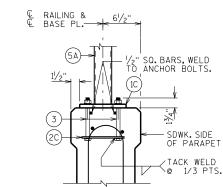
1/4" \$ VENT HOLE. FACE TO OUTSIDE OF RAIL.

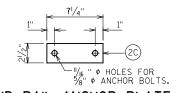
PARAPET





END RAIL BASE PLATE





END RAIL ANCHOR PLATE FOR END RAIL BASE PLATES 2 REO'D. PER END RAIL BASE PLATE



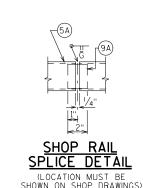


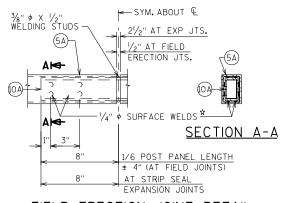
— II/₁₆

(2 SETS PER POST)

-1/16" THK.

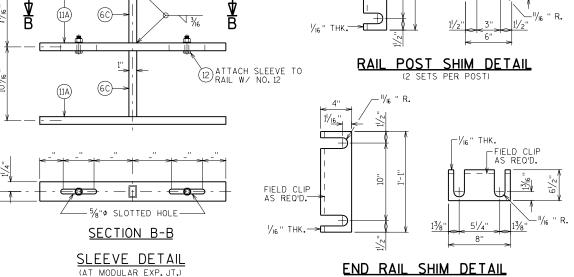
FIELD CLIP AS REQ'D.





FIELD ERECTION JOINT DETAIL

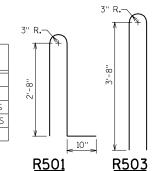
☆MIN. 5/8" FLAT SURFACE DIA. PUNCHINGS OR STUDS MAY BE USED AS AN ALTERNATE.



FIFLD CLIP

AS REQ'D.

BAR MARK	C047	ABUT.	ABUT.	LENGTH	MY	LOCATION
R501	Х			6-10	Х	PARAPET VERT.
R402	Х			-		PARAPET HORIZ.
R503	Х			8-2	Х	PARAPET VERT.@ WINGS
R404	Х			-		PARAPET HORIZ.@ WINGS



LEGEND

- (1A) PLATE $\frac{5}{8}$ " X 6" X 8" WITH $\frac{3}{4}$ " X $\frac{1}{2}$ " SLOTTED HOLES.
- (1C) PLATE 5/8" X 8" X 1'-1" WITH 3/4" X 11/2" SLOTTED HOLES.
- $(2A)^{1}/4$ " X 5" X 7" ANCHOR PLATE WITH $\frac{1}{16}$ " ϕ HOLES FOR ANCHOR BOLTS NO. 3.
- 3 5%" DIA. X 71/2" LONG ASTM F593 TYPE 316 STAINLESS STEEL ANCHOR BOLTS WITH NUT AND WASHERS OF SAME ALLOY GROUP. (ALTERNATE RAIL POST ANCHORAGE 4 EQUIV. STAINLESS STEEL CONCRETE MASONRY ANCHORS, TYPE S (EPOXY), 5%" 0, MINIMUM PULLOUT CAPACITY OF 15 KIPS. EMBED A MIN. OF 7" FOR RAIL POSTS AND 5" FOR END RAILS.)
- $\stackrel{(4)}{\longrightarrow}$ STRUCTURAL TUBING 3" X $11\!/_2$ " X $^3\!/_6$ ". PLACE VERTICAL. WELD TO NO.1 & 5.
- (5A) STRUCTURAL TUBING 3" X $1\frac{1}{2}$ " X $\frac{3}{6}$ " RAILS. WELD TO NO.1 & NO.4.
- (6B) STRUCTURAL TUBING 1" X 1½" X ½" PICKETS. WELD TO NO.5. SPACE AT 6" MAX. \pounds TO \pounds SPACING. PLACE VERTICAL.
- 60 STRUCTURAL TUBING 1" X $1\!/_{\!2}$ " X $1\!/_{\!8}$ " PICKETS, WELD TO NO.11, PLACE VERTICAL.
- (8)5" ϕ SCH. 40 PIPE (5% 0.D.) $1\frac{1}{2}$ " LONG SLICES. WELD TO NO. 5A.
- 9A) RECTANGULAR SLEEVE FABRICATED FROM 36" PLATES. PROVIDE
- (0) RECTANGULAR SLEEVE FABRICATED FROM 36" PLATES. (1'-4" @ FIELD ERECTION JTS.) (1'-4" @ STRIP SEAL EXP. JTS.)
- (11A) BAR 21/2" X 1" X '- ".
- (12) 1/2" DIA. STAINLESS STEEL BOLT WITH NUT AND LOCKWASHER.

NOTES

BID ITEM SHALL BE "COMBINATION RAILING TYPE "C1" ", WHICH SHALL INCLUDE ALL STEEL ITEMS SHOWN, AND PAINTING.

POST BASE PLATES SHALL BE FLAT WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL. ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS.

NO.1, 2, 8, 9 AND NO.10 SHALL CONFORM TO THE REQUIREMENTS OF A.S.T.M. A709 CRADE 36. STRUCTURAL TUBING SHALL CONFORM TO THE REQUIREMENTS OF A.S.T.M. A500 GRADE B (NO. 4, NO. 5, AND NO. 6).

ANCHORAGES SHALL BE ACCURATELY PLACED TO PROVIDE CORRECT ALIGNMENT OF RAILING, SET NORMAL TO GRADE.

CUT BOTTOM OF POST TO MAKE POST VERTICAL IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTION.

STEEL SHIMS SHALL BE PROVIDED & USED UNDER BASE PLATES WHERE REQUIRED FOR

FILL BOLT SLOT OPENINGS IN SHIMS AND PLATE NO.1AND CAULK AROUND PERIMETER OF PLATE NO.1WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER.

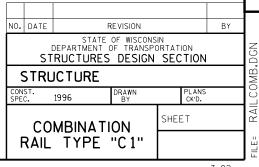
ALL JOINTS AND RECESSES IN CONCRETE PARAPET ARE TO BE VERTICAL.

AFTER FABRICATION, ALL MATERIAL, EXCEPT ANCHORAGE (NO. 2 & 3) & SHIMS SHALL BE PAINTED WITH A THREE COAT ZINC-RICH EPOXY SYSTEM PER WISDOT STANDARD SPECIFICATION, SECTION 517, EPOXY SYSTEM, SHIMS SHALL BE GIVEN ONE COAT OF ZINC RICH PRIMER PAINT. THE FINISH COLOR SHALL BE FEDERAL COLOR NO. . .

 $^{1}\!/_{\!4}$ ' vent holes located in top rail over rail posts and at low end of other rails.

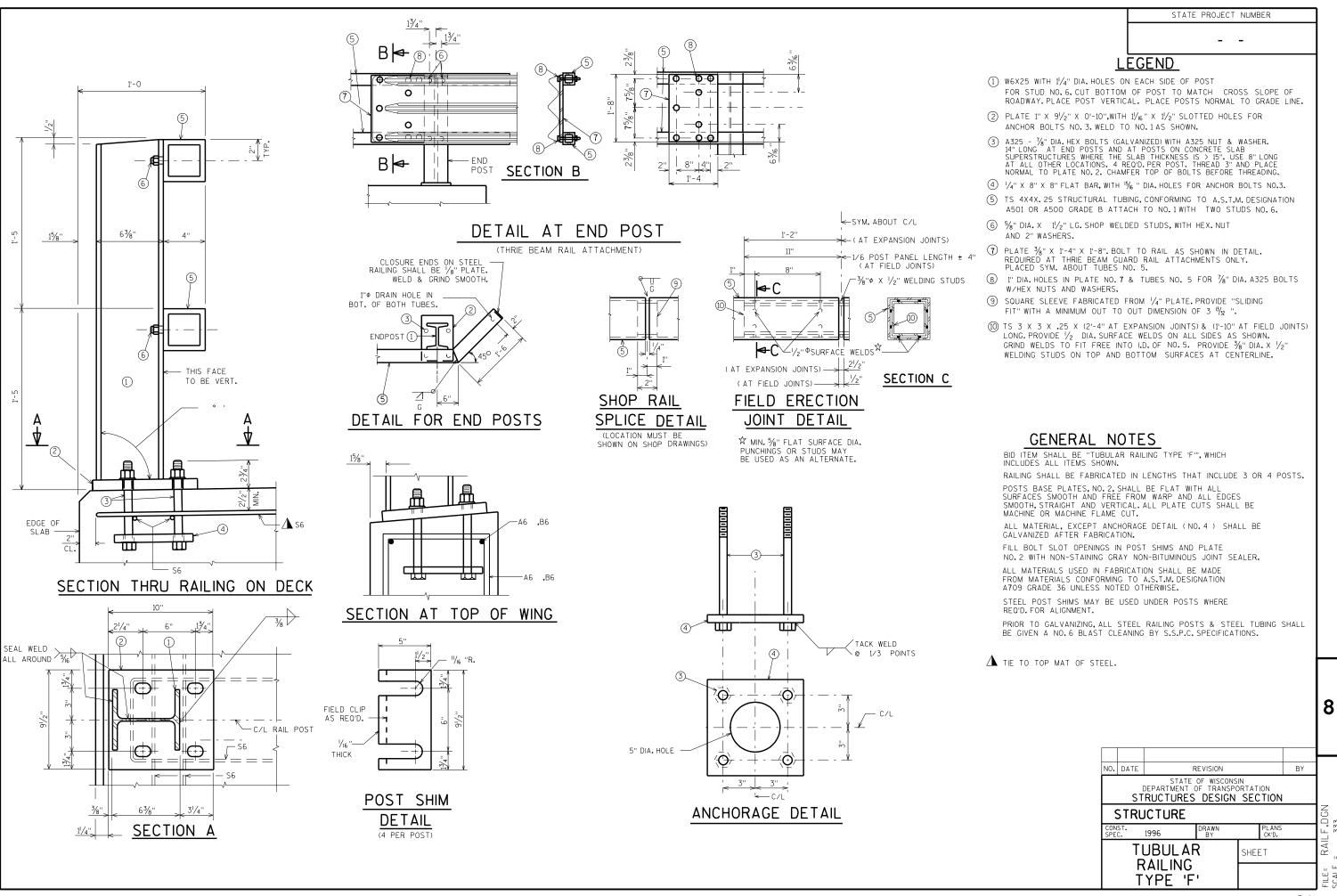
RAILING SHALL BE FABRICATED IN LENGTHS THAT INCLUDE 3 OR 4 POSTS.

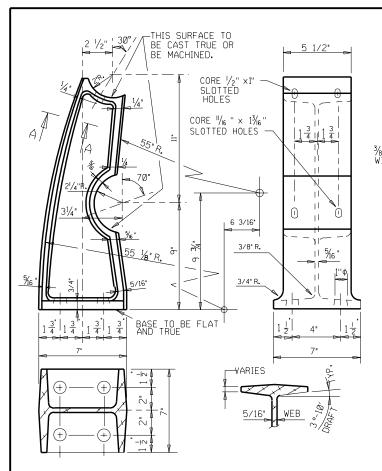
TOUCH-UP PAINTING TO BE DONE AT COMPLETION OF STEEL RAILING INSTALLATION TO THE SATISFACTION OF THE ENGINEER AT NO EXTRA COST.



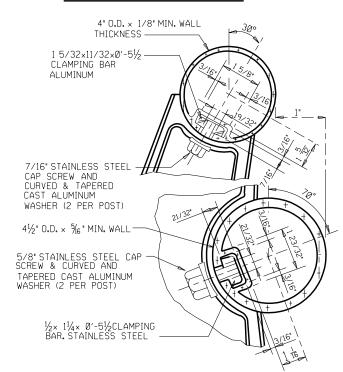
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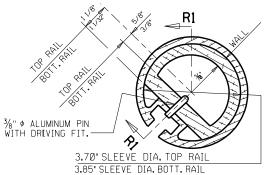
ALUMINUM POST CASTING



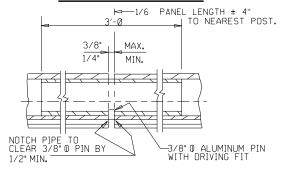
DETAIL OF RAIL ATTACHMENT TO POST

NOTES: MAX. REDUCTION IN DIAMETER OF BENT SECTION SHALL BE 3% WALL THICKNESS OF TUBING SHOWN ABOVE SHALL BE MIN. NOMINAL AVERAGE WALL THICKNESS.

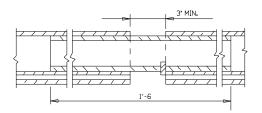
MAX. REDUCTION IN SLOT WIDTH IN BENT TUBING SHALL BE 3/16*.



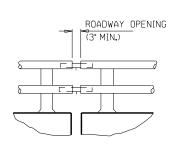
RAIL SPLICE DETAIL



SECTION R1

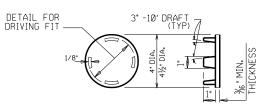


SLEEVE DETAIL AT ABUTMENT

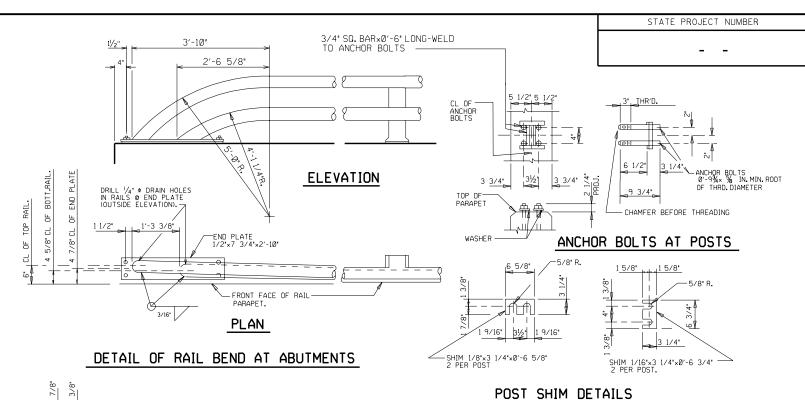


DETAIL AT RAIL OPENINGS

ALL SLEEVE DETAILS SAME AS "RAIL SPLICE DETAIL"
UNLESS SHOWN OTHERWISE



RAIL CLOSURE CAP DETAIL



1 1/4" DIA, DRILLED HOLES

CHAMFER BEFORE THREADING

FOR PRESET ANCHOR BOLTS.

6 1/2"

9 3/4"

ANCHOR BOLTS-0'-9 3/4"LONG.

FUST SHIM DETHILS

GENERAL NOTES

BID ITEM SHALL BE TUBULAR RAILING, TYPE "H" WHICH INCLUDES ALL ITEMS SHOWN.

THE SHANK AND ROOT DIAMETER OF THREAD FOR ANCHOR BOLTS SHALL BE A MIN. OF $5/\!\!\!/_{8}$ ".

ANCHOR BOLTS, NUTS AND WASHERS SHALL BE STAINLESS STEFI.

SHIMS SHALL CONFORM TO SAME MATERIAL AS POSTS.

RAILINGS SHALL BE FABRICATED IN 2 AND 3 PANEL LENGTHS.

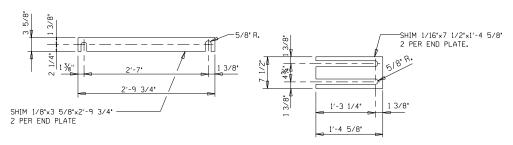
RAILING POSTS SHALL BE SET NORMAL TO GRADE LINE.

ALL POST SPACINGS ARE MEASURED HORIZONTALLY ALONG CENTERLINE OF THE POST BASE.

SHIMS SHALL BE USED UNDER POSTS AND END PLATES WHERE REO'D. FOR ALIGNMENT.

FILL ALL EXPOSED OPENINGS BETWEEN SHIMS AND POST ANCHOR BOLT HOLES WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER.

RAILS SHALL BE BUILT STRAIGHT AND SPRUNG INTO PLACE FOR STRUCTURES CURVED UP TO 3°, FOR STRUCTURES CURVED GREATER THAN 3°, RAILS SHALL BE CURVED TO FIT.



END PLATE SHIM DETAILS

2'-10"

3/4" SQ. BARS

WASHER

7 1/2' LONG.

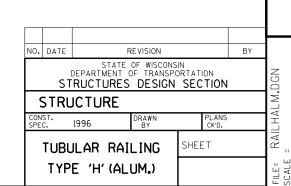
TOP OF

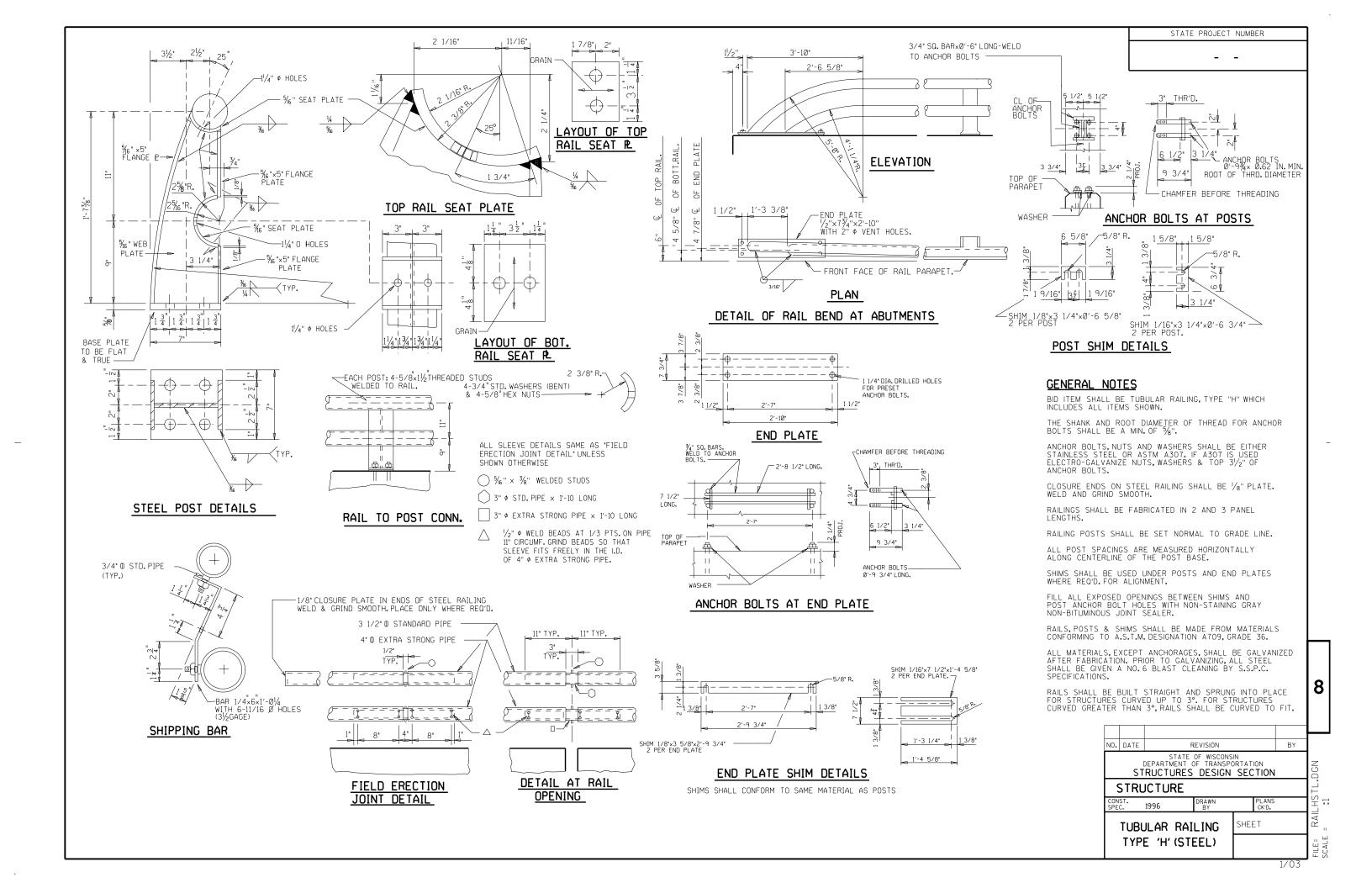
WELD TO ANCHOR BOLTS. 7 END PLATE

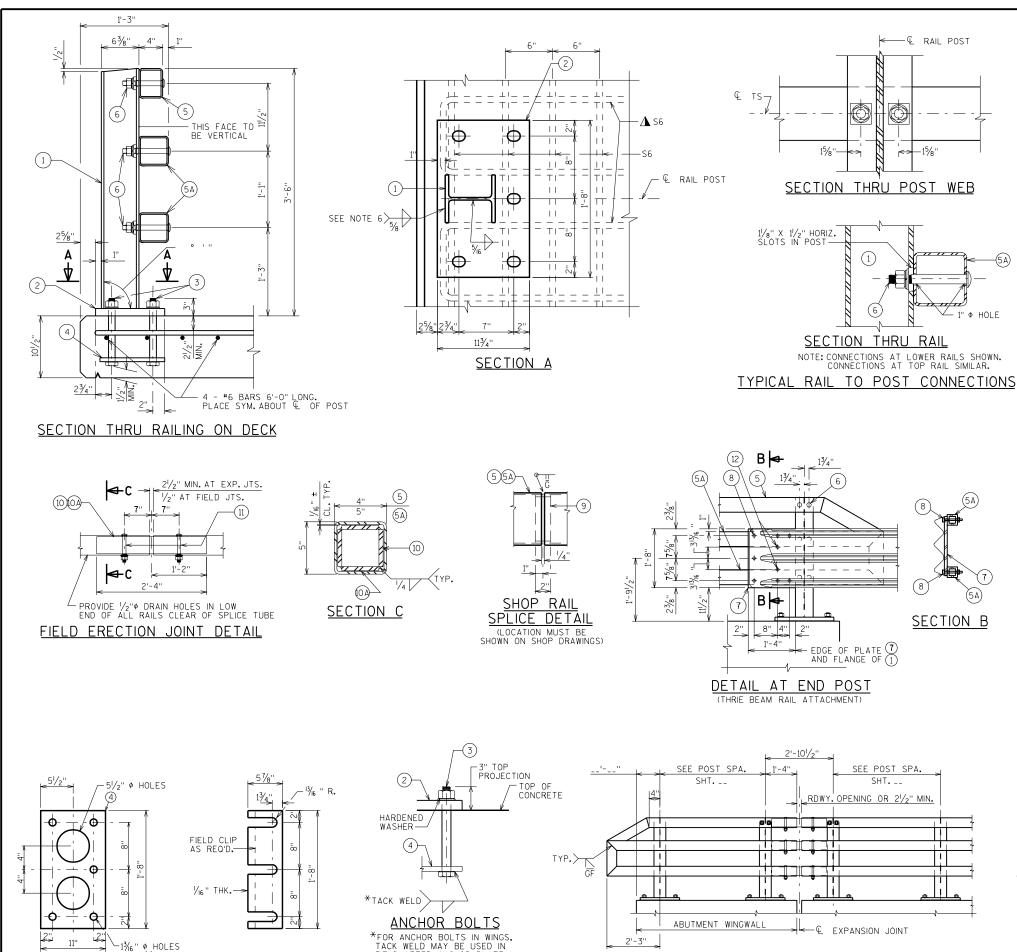
_2'-8 1/2" LONG.

ANCHOR BOLTS AT END PLATE

SHIMS SHALL CONFORM TO SAME MATERIAL AS POSTS







FIELD AFTER ANCHOR PLATE IS IN POSITION IF REQ'D. FOR

CONSTRUCTIBILITY.

' Φ HOLES FOR 11/8" P

POST SHIM DETAIL

ANCHOR PLATE

LEGEND

 $\stackrel{\hbox{\scriptsize (1)}}{}$ W6 x 25 With 1½" X 1½" Horiz, slots on each side of Post for Bolt No.6. Cut Bottom of Post to Match cross slope of Roadway. Place Post Vertical, place Posts Normal to Grade line.

STATE PROJECT NUMBER

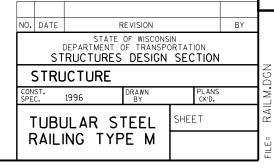
- ② PLATE 11/4" × $11\frac{3}{4}$ " × 1^{1} -8" WITH $1\frac{5}{6}$ " X $1\frac{5}{8}$ " SLOTTED HOLES FOR ANCHOR BOLTS NO. 3. WELD TO NO. 1 AS SHOWN. SLOTS PARALLEL TO SHORT SIDE OF PLATE.
- 3 ASTM A449 1½" DIA. ANCHOR BOLTS WITH NUT AND HARDENED WASHER (ALL GALVANIZED). 5 REO'D, PER POST. THREAD 3" AND PLACE NORMAL TO PLATE NO. 2. CHAMFER TOP OF BOLTS BEFORE THREADING. USE 1-9" LONG IN ABUTMENT WINGS. AT POSTS ON CONCRETE SLAB SUPERSTRUCTURES WHERE THE SLAB THICKNESS IS > 16" USE 1'-3" LONG. USE 10¾" LONG AT ALL O'THER LOCATIONS. (AN EQUIVALENT THREADED ROD WITH NUTS AND HARDENED WASHERS MAY BE SUBSTITUTED FOR ANCHOR BOLTS IN WINGS IF REO'D. FOR CONSTRUCTIBILITY.)
- $\textcircled{4}~\%"\times11"\times1"\text{-8"}$ Anchor plate (Galvanized) with $1\%_6"$ Dia. Holes for anchor Bolts no. 3
- (5) TS 5 \times 4 \times 0.25 STRUCTURAL TUBING. ATTACH TO NO.1 WITH NO.6.
- (5A) TS 5 \times 5 \times 0.25 STRUCTURAL TUBING. ATTACH TO NO.1 WITH NO.6.
- \bigcirc Plate $\frac{3}{4}$ " \times 1'-4" \times 1'-8". Bolt to rail as shown in detail, required at three Beam guard rail attachments only, place symmetrically about TUBES NO. 5A.
- 8 1" DIA. HOLES IN PLATE NO. 7 & TUBES NO. 5A FOR %" DIA. A325 BOLTS WITH HEX NUTS AND WASHERS. 6 HOLES IN TUBES AND PLATE NO. 7.
- 9 SPLICE SLEEVE FABRICATED FROM 1/4" PLATE. PROVIDE "SLIDING FIT".
- (10) 3/8" X 35/8" X 2'-4" PLATE. 2 PER RAIL. USED IN NO. 5 & 5A.
- (0) %" X 2%" X 2'-4" PLATE USED IN NO.5, %" X 3%" X 2'-4" PLATE USED IN NO.5A. 2 PER RAIL.
- (1) 7_8 " ϕ A325 ROUND HEAD BOLT WITH NUT, WASHER, AND LOCK WASHER. USE $1\%_6$ " X $1^1\!/4$ " LONGIT. SLOTTED HOLES AT FIELD JOINTS AND $1\%_6$ " X $2^1\!/4$ " MIN. LONGIT. SLOTTED HOLES AT EXP. JOINTS IN PLATE NO. 10A.
- (12) $\frac{7}{8}$ " DIA. X $\frac{1}{2}$ " LONG THREADED SHOP WELDED STUDS (3 REQ'D).

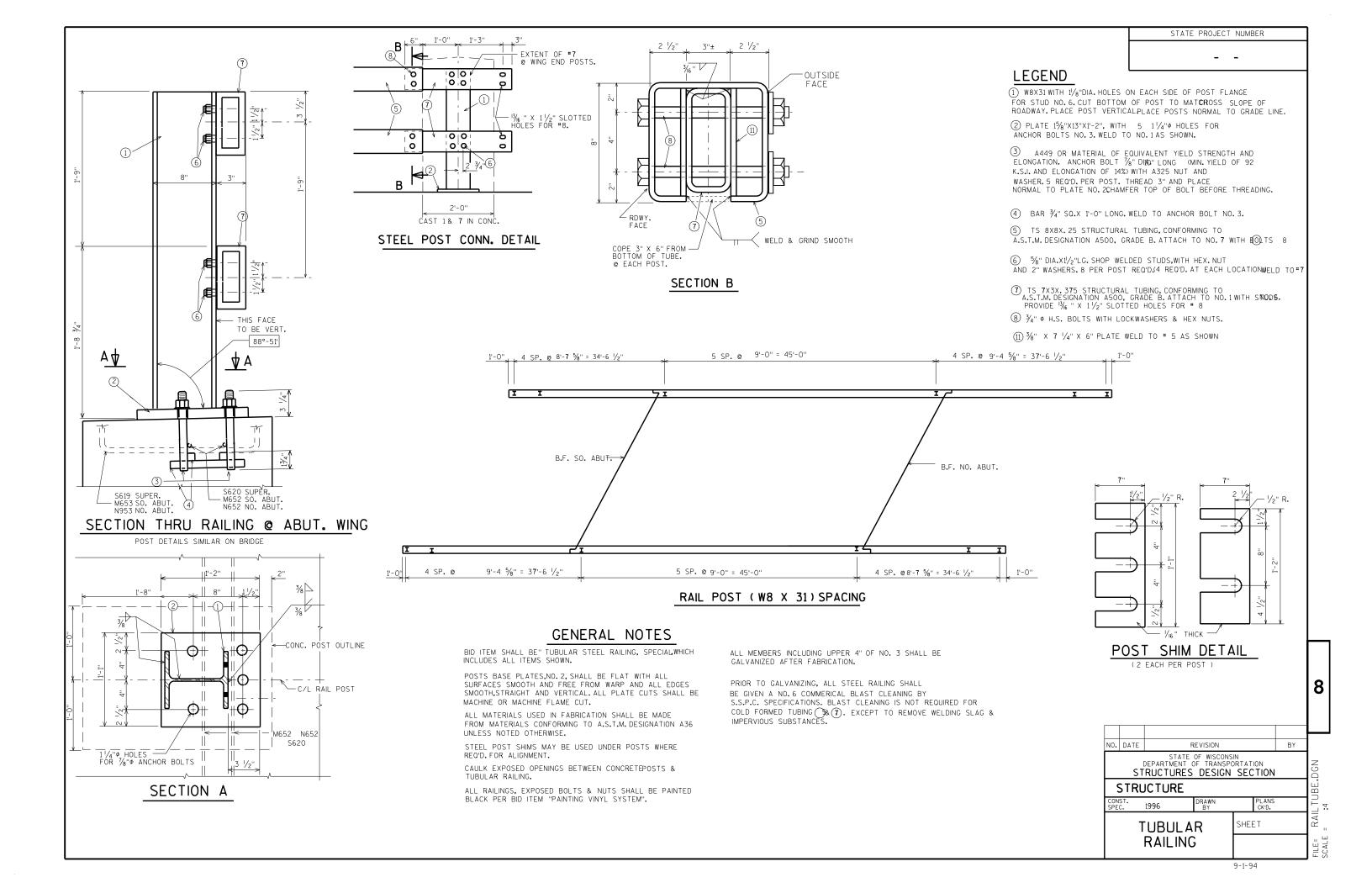
GENERAL NOTES

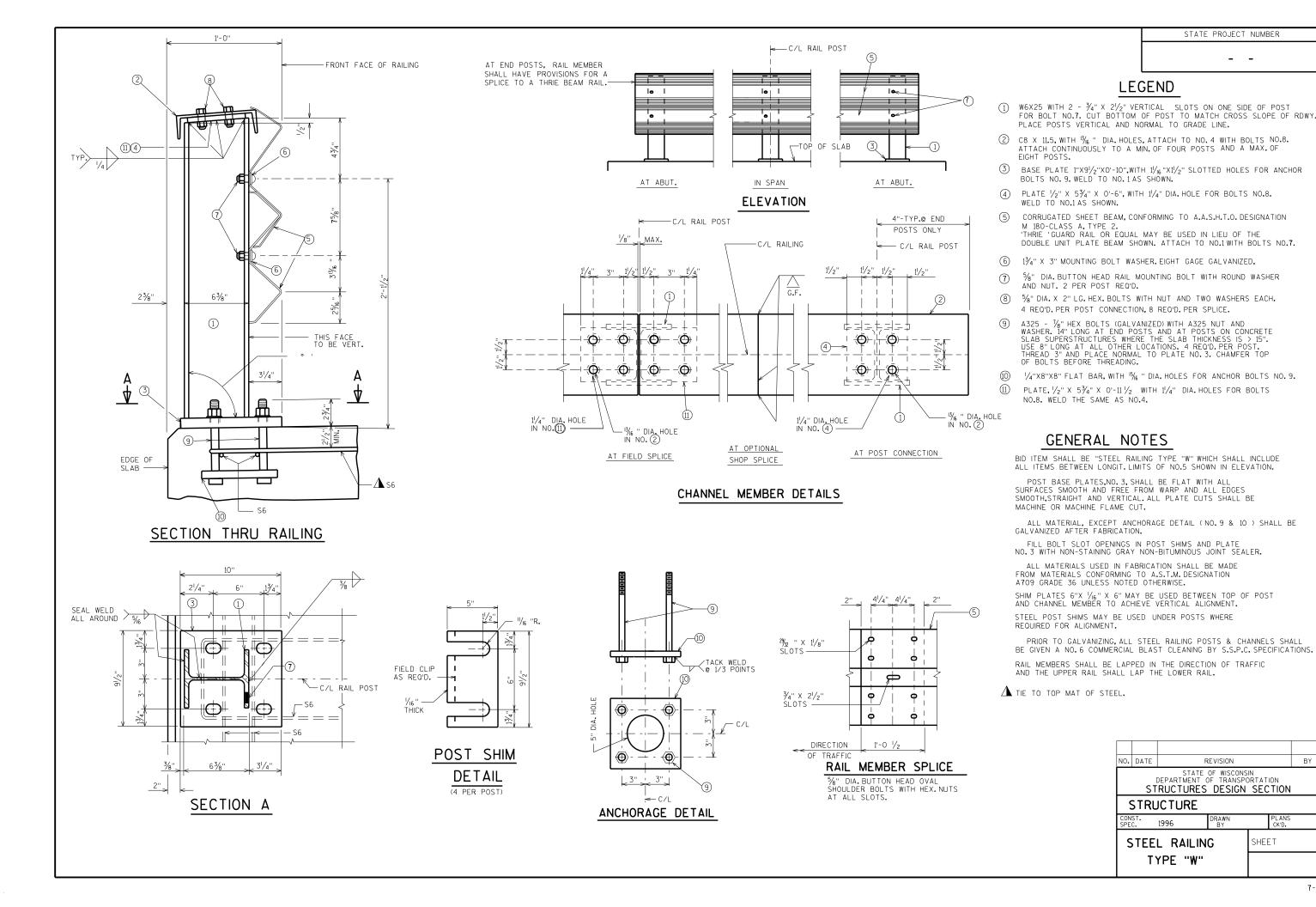
- 1. BID ITEM SHALL BE "TUBULAR RAILING TYPE "M" WHICH INCLUDES ALL ITEMS SHOWN.
- 2. RAIL POST AND BASE PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 50. HOLLOW RAILING STRUCTURAL TUBING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500 GRADE B OR C WITH A CERTIFIED FY = 50 KSI. ANCHOR PLATES, AND SPLICE TUBE PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 36.
- 3. THE NUT SECURING THE POST BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN AN ADDITIONAL ${}^{\prime}\!/_8$ TURN.
- 4. RAILS SHALL BE CONTINUOUS OVER A MINIMUM OF FOUR (4) POSTS WITHOUT SPLICES WHERE POSSIBLE. RAILS SHALL BE SPLICED IN A PANEL OVER EXPANSION JOINTS.
- 5. ENDS OF TUBE SECTIONS SHALL BE SAWED. GRIND SMOOTH EXPOSED EDGES. ALL CUT ENDS SHALL BE TRUE AND SMOOTH.
- 6. WELD IS THE SAME ON BOTH FLANGES, FLANGE WELD DOES NOT REQUIRE MAGNETIC PARTICLE
- 7. FILL BOLT SLOT OPENINGS IN POST SHIMS AND PLATE NO.2 AND CAULK AROUND PERIMETER OF PLATE NO.2 WITH NON-STAINING GRAY NON-BITUMINOUS JOINT SEALER. STEEL POST SHIMS MAY BE USED UNDER POSTS WHERE REO'D. FOR ALIGNMENT.
- 8. POST BASE PLATES SHALL BE FLAT WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL. ALL PLATE CUTS SHALL BE MACHINE OR MACHINE
- 9. FOR RAILING NOT TO BE PAINTED, ALL MATERIAL SHALL BE GALVANIZED AFTER FABRICATION. PRIOR TO GALVANIZING, ALL STEEL RAILING POSTS & STEEL TUBING SHALL BE GIVEN A NO. 6 BLAST CLEANING BY S.S.P.C. SPECIFICATIONS.
- 10. FOR RAILING TO BE PAINTED, ALL MATERIAL EXCEPT ANCHORAGE DETAIL (NO. 3 & 4) SHALL BE PAINTED WITH A THREE-COAT ZINC RICH EPOXY SYSTEM. PRIOR TO PAINTING, ALL STEEL RAILING POSTS & STEEL TUBING SHALL BE GIVEN A NO. 11 NEAR WHITE BLAST CLEANING BY
- 11. THIS RAILING MEETS NCHRP REPORT 350 EVALUATION CRITERIA FOR TEST LEVEL 4 (TL-4).

A TIE TO TOP MAT OF STEEL.

PART ELEVATION OF RAILING

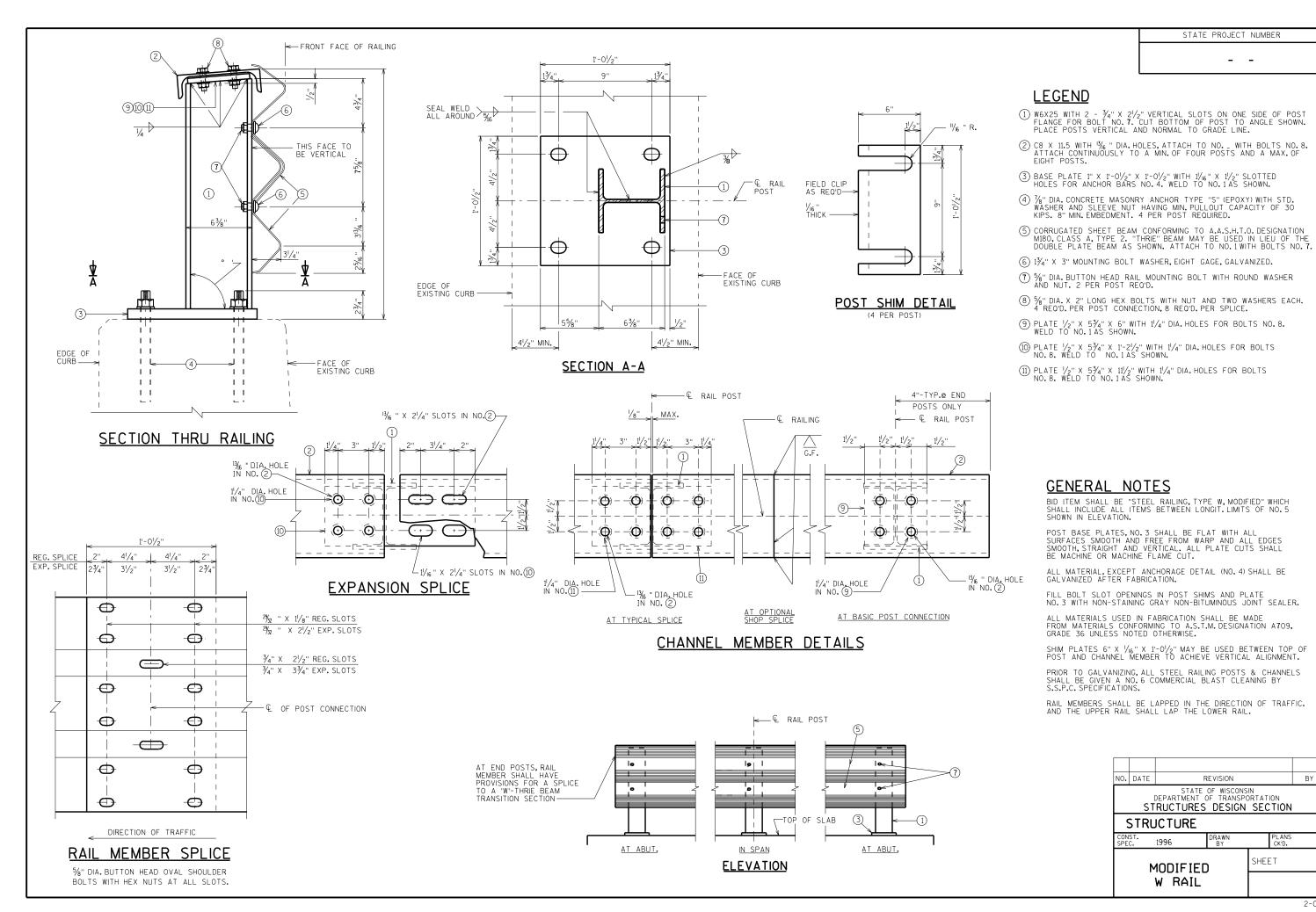






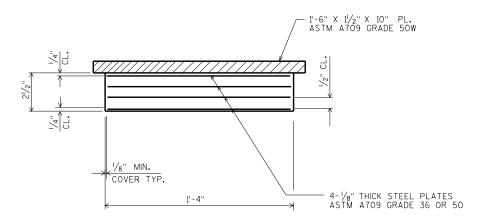
BY

SHEET



STATE PROJECT NUMBER

- SPACE SIX 5/8" ¢ X 63/8" LONG STUDS TO CLEAR PRESTRESSING STRANDS — L € BEARING — € GIRDER PRESTRESSED GIRDERS END OF-GIRDER ELASTOMERIC STAINLESS STEEL BEVELED ANCHOR PLATE (ASTM A240, TYPE 304) CAST TO GIRDER -1.1 1.1 BEARING 1.1 1.1 1.1 USE E309 ELECTRODE 1.1 1.1 1 1 - STEEL PLATE (ASTM A709 GRADE 50W) NO. 6 BLAST. VULCANIZE PLATE TO ELASTOMERIC PAD. -STEEL LAMINATED ELASTOMERIC BEARING (DUROMETER 55 ± 5) 11/2" TYP. 11/2" STEEL PLATE AND STAINLESS STEEL BEVELED ANCHOR PLATE – ELASTOMERIC BEARING PLAN VIEW END VIEW



SECTION THRU ELASTOMERIC BEARING

BEARING NOTES

BEARINGS SHALL NOT BE PLACED AT A TEMPERATURE GREATER THAN 85° F.

ALL MATERIAL USED FOR BEARINGS SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "LAMINATED ELASTOMERIC BEARING PADS, EACH".

ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL

ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS.

NO. DATE REVISION BY

STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

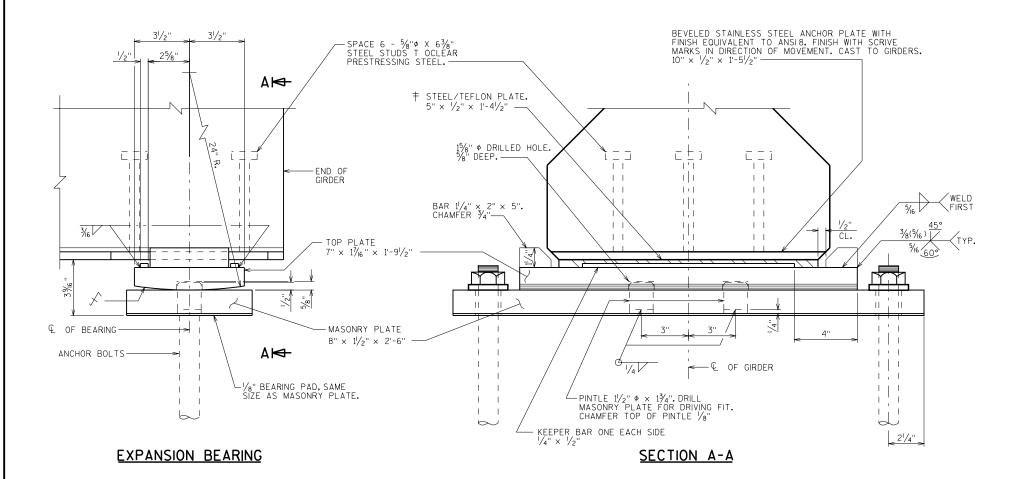
CONST. 1996 DRAWN
BY

PLANS
CK.D.

PRESTRESSED SHEET

GIRDER BEARINGS

4



BEARING NOTES

ALL BEARINGS ARE SYMMETRICAL ABOUT & OF GIRDER AND & OF BEARING.

ALL MATERIAL INCLUDING SHIMS, BUT EXCLUDING STAINLESS STEEL PLATE, TEFLON SURFACE, PINTLES, ANCHOR BOLTS, NUTS AND WASHERS SHALL CONFORM TO ASTM A709 GRADE 50W.

STAINLESS STEEL PLATE SHALL CONFORM TO A.S.T.M. A240, TYPE 304.

STEEL PINTLES SHALL CONFORM TO ASTM A449 OR MATERIAL OF EQUIVALENT YIELD STRENGTH AND ELONGATION.

FABRICATOR MAY INCREASE "MASONRY PLATE" THICKNESS AS AN ALTERNATE TO SHIMS.

ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED STEEL PLATES WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT, AND VERTICAL.

ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS.

ALL SURFACES MARKED & SHALL BE MACHINE FINISHED ANSI 250 UNLESS OTHERWISE SHOWN.

ALL FINISHED SURFACES SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.

ALL ANCHOR BOLTS, NUTS AND WASHERS SHALL CONFORM TO ASTM A709 GRADE 36, OR MATERIAL OF EQUIVALENT YIELD STRENGTH AND ELONGATION.

CHAMFER ANCHOR BOLTS PRIOR TO THREADING.

MASONRY PLATE, TOP PLATE, KEEPER BARS, ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AS REQUIRED BY ASTM DESIGNATION A153, CLASS "C". STEEL PLATE ATTACHED TO TEFLON SURFACE SHALL BE SHOP PAINTED.

ANCHOR BOLTS SHALL BE THREADED 3". PROVIDE ONE STANDARD WROUGHT WASHER AND ONE HEX NUT PER BOLT. PROJECT ANCHOR BOLTS "MASONRY PLATE" THICKNESS $+2^1/4$ " ABOVE TOP OF CONCRETE.

ALL MATERIALS IN "STEEL BEARINGS FOR PRESTRESSED CONCRETE GIRDERS", INCLUDING SHIMS, SHALL BE PAID FOR AT THE UNIT PRICE BID FOR EITHER "EXPANSION BEARING ASSEMBLIES, EACH".

- * DRILLED HOLES FOR ANCHOR BOLTS SHALL HAVE A DIAMETER $\frac{3}{8}$ " LARGER THAN ANCHOR BOLT.
- # TEFLON SURFACE, USE UNFILLED MIN. 1/6" THICKNESS. PLACE WITH SCRIVE MARKS IN DIRECTION OF MOVEMENT. BOND STEEL AND TEFLON WITH ADHESIVE MATERIAL MEETING FED. SPEC. MMM-A-134, FEP FILM OR EQUAL.

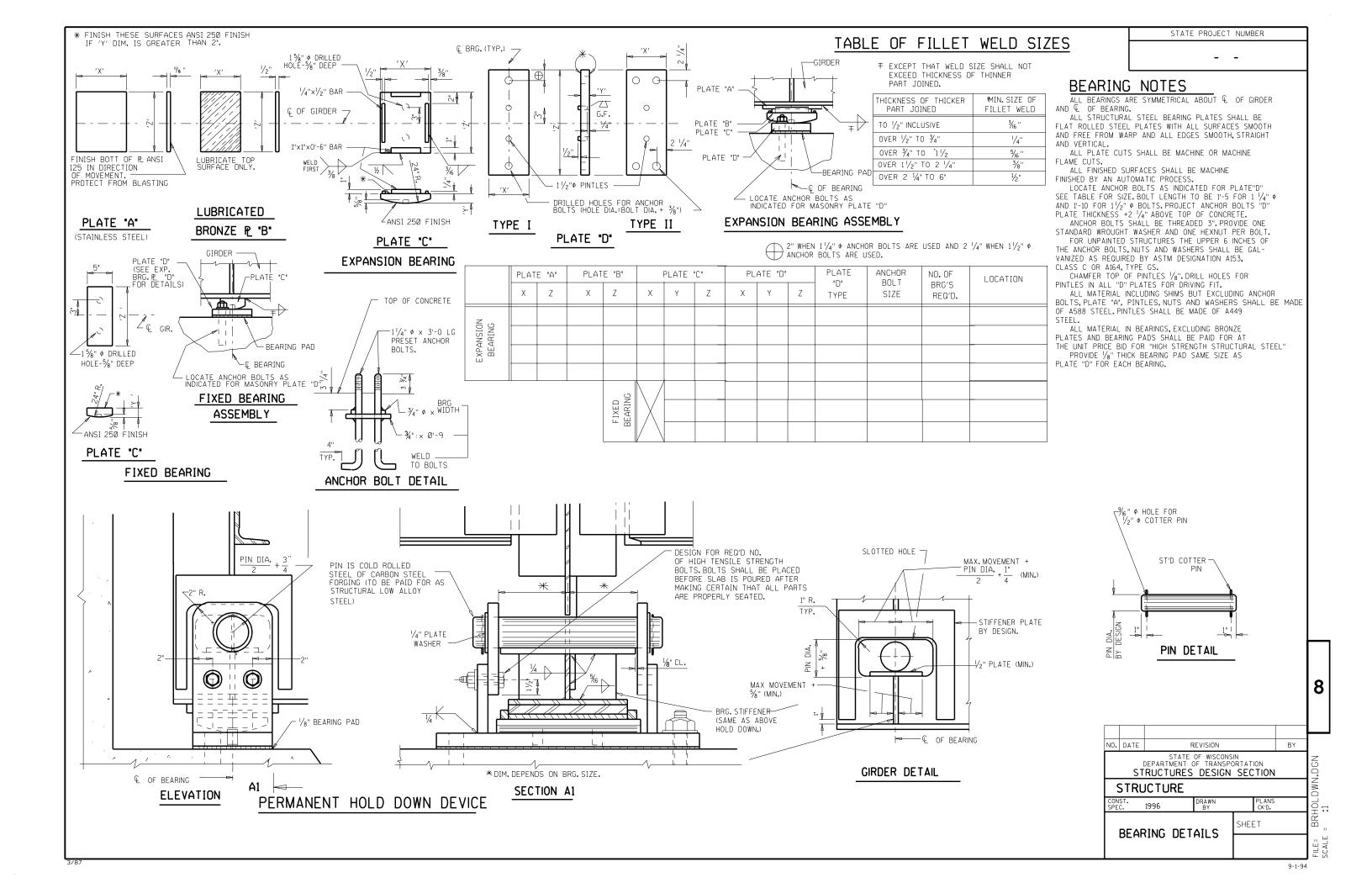
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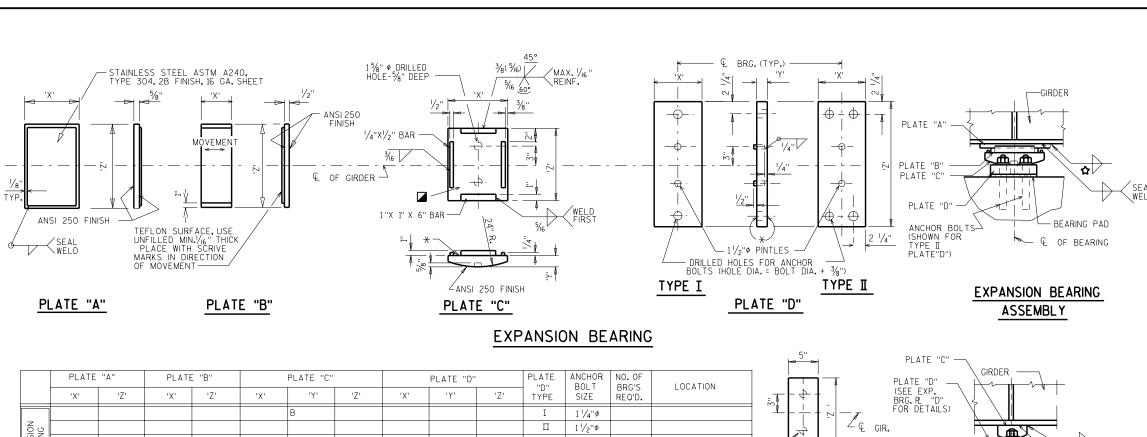
STATE OF WISCONSIN
DEPARTMENT OF TRANSPORTATION
STRUCTURES DESIGN SECTION

STRUCTURE

CONST. 1996 DRAWN
BY PLANS
CKID.

PRESTRESSED
GIRDER STEEL
BEARINGS





—1 %" φ DRILLED HOLE, 5/8" DEEP ANCHOR BOLTS (SHOWN FOR BEARING PAD TYPE I PLATE"D") ← Q OF BEARING

> FIXED BEARING **ASSEMBLY**

FIXED BEARING

– ANSI 250 FINISH

PLATE "C"

ANSI 250 FINISH

BEVELED PLATE "C"

BEARING NOTES

ALL BEARINGS ARE SYMMETRICAL ABOUT & OF GIRDER AND & OF BEARING.

ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED STEEL PLATES WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL.

ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS. ALL FINISHED SURFACES SHALL BE MACHINE FINISHED BY AN AUTOMATIC PROCESS.

STATE PROJECT NUMBER

CHAMFER ANCHOR BOLTS PRIOR TO THREADING. ANCHOR BOLTS SHALL BE THREADED 3". PROVIDE ONE STANDARD WROUGHT WASHER AND ONE HEX NUT PER BOLT. BOLT LENGTH TO BE 1'-5 FOR 1 $\frac{1}{4}$ " ϕ AND 1'-10 FOR 1 $\frac{1}{2}$ " ϕ BOLTS. PROJECT ANCHOR BOLTS "D"PLATE THICKNESS +2 1/4" ABOVE TOP OF CONCRETE.

CHAMFER TOP OF PINTLES 1/8 ". DRILL HOLES FOR PINTLES IN ALL "D" PLATES FOR DRIVING FIT.

ALL MATERIAL INCLUDING SHIMS BUT EXCLUDING ANCHOR BOLTS, STAINLESS STEEL, TEFLON SURFACE, PINTLES, NUTS AND WASHERS SHALL BE MADE OF ASTM A709 GRADE 50W. STEEL PINTLES SHALL BE MADE OF ASTM A449 STEEL OR MATERIAL OF EQUAL YIELD STRENGTH & ELONGATION. ANCHOR BOLTS, NUTS & WASHERS SHALL CONFORM TO ASTM A709 GRADE 36 OR MATERIAL OF EQUIV. STRENGTH & ELONGATION.

PROVIDE $\frac{1}{8}$ " THICK BEARING PAD SAME SIZE AS PLATE "D" FOR EACH BEARING.

ALL MATERIAL IN BEARINGS, INCLUDING BEARING PADS & SHIM PLATES SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "EXPANSION BEARING ASSEMBLIES" OR "FIXED BEARING ASSEMBLIES", RESPECTIVELY.

ANCHOR BOLTS, NUTS & WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C.

ALL SURFACES OF BEARING PLATES, EXCEPT STAINLESS STEEL & TEFLON SURFACES, SHALL BE PAINTED AFTER GALVANIZING WITH A TIE COAT, INTERMEDIATE COAT & TOP COAT. FOR UNPAINTED STRUCTURES, BEARING PLATES WELDED TO THE GIRDER NEED NOT BE PAINTED.

- * FINISH THESE SURFACES ANSI 250 FINISH IF 'Y' DIM. IS GREATER THAN 2".
- PROVIDE A METHOD FOR HANDLING PLATE "C" DURING GALVANIZING.

TABLE OF FILLET WELD SIZES

THICKNESS OF THICKER PART JOINED	≠ MIN.SIZE OF FILLET WELD
TO 1/2" INCLUSIVE	3/16 ''
OVER 1/2" TO 3/4"	1/4"
OVER 3/4" TO 11/2"	5/16 ''
OVER 1 1/2" TO 2 1/4"	3/8"
OVER 2 1/4" TO 6"	1/2"
01211 2 /4 10 0	/ 2

EXCEPT THAT WELD SIZE SHALL NOT EXCEED THICKNESS OF THINNER PART JOINED.

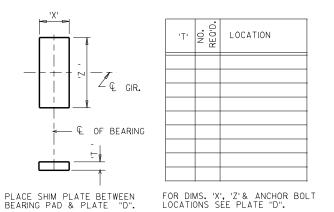
	 - -
3/8''	OR COVER PLATE
MIN.	

BEARING REPLACEMENT DETAILS

3⁄4"¢ ROD _ HEX NUT THR'D 4" - BEVELED OR PLATE WASHER - 15/6" X 11/2" SLOTTED HOLE IN END DIAPH. (SLOTTED IN DIRECTION OF CHANNEL) STD. GAGE ∠AFTER SUPERSTRUCTURE CONCRETE IS POURED BURN OFF BAR FLUSH WITH CONCRETE SURFACE

TEMPORARY HOLD DOWN

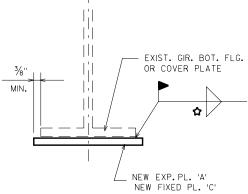
PLACE ONE PER GIRDER AT ABUTMENT WHERE SLAB POUR TERMINATES. LOCATE 1'-6 (NORMAL) OFF € OF GIRDER. TO BE PAID FOR AS "STRUCTURAL CARBON STEEL".



SHIM PLATE DETAILS

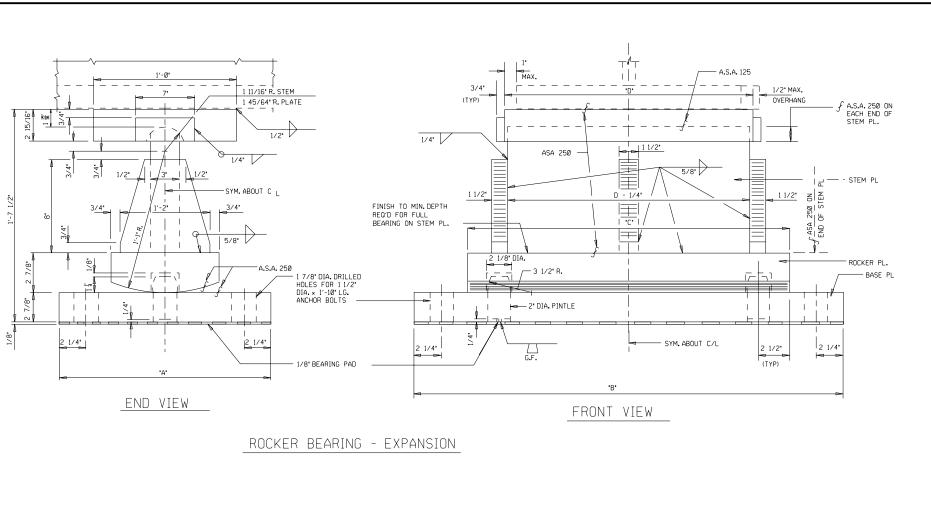
IN LIEU OF SHIM PLATE, THICKNESS OF PLATE "A" OR "D". MAY BE INCREASED BY "T" THE SHIM PLATE THICKNESS.

SHIM PLATE NOT REQUIRED IF FLANGE BUTT SPLICE IS ELIMINATED & THE LARGER FLANGE PLATE IS EXTENDED TO THE END OF THE GIRDER.



NO. DATE BY STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION STRUCTURE HEET BEARING DETAILS

TEMP HOLD DOWN = LV 3



1/4"

FINISH TO MIN. DEPTH

REO'D. FOR FULL BEARING ON STEM P

— A.S.A. 125

5/8"

- A.S.A. 1000

SYM. ABOUT C L-

FRONT VIEW

7 _1/2" MAX. OVERHANG

12

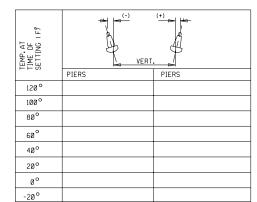
1 1/2"

- ∫ A.S.A.250 ON EACH END OF STEM PL

STEM Pi

2 1/4

- BASE PL



GENERAL NOTES

STATE PROJECT NUMBER

ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED STEEL PLATES WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL.

ALL PLATE CUTS SHALL BE MACHINE OF MACHINE FLAME CUTS.

PAY QUANTITIES FOR SIDE PLATES ON BEARINGS WILL BE BASED ON ACTUAL SIZE.

PINTLES SHALL BE MACHINED TO A DRIVING FIT.

ANCHOR BOLTS SHALL BE THREADED 3. PROVIDE ONE $1/4^{\circ}$ WASHER AND ONE HEAVY HEX.NUT PER ANCHOR BOLT.

ALL MATERIAL FOR BEARINGS AT PIERS, EXCEPT NUTS, BOLTS AND WASHERS, SHALL CONFORM TO ASTM SPECIFICATION TYPE A709 GRADE 50W

ALL NUTS AND BOLTS SHALL CONFORM TO ASTM SPECIFICATION TYPE A709, GRADE 36.

PINTLES SHALL BE MADE OF ASTM A449 STEEL.

ALL SURFACES MARKED f SHALL BE MACHINE FINISHED BY AUTOMATIC PROCESS.

RADIAL SURFACES ON ROCKER SHALL BE MACHINE FINISHED AFTER WELDING.

AFTER WELDING FIXED SHOE ASSEMBLY, FINISH BOTTOM OF BASE PLATE TO A FLAT SURFACE.

ALL MATERIAL IN BEARING, EXCLUDING BEARING PADS, SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "HIGH STRENGTH STRUCTURAL STEEL".

PROVIDE A 1/8" THICK BEARING PAD SAME SIZE AS THE BASE PLATE FOR EACH BEARING.

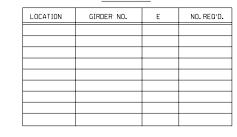
ANCHOR BOLTS TO PROJECT THICKNESS OF BASE PLATE +2 1/4' ABOVE TOP OF CONCRETE.

FOR UNPAINTED STRUCTURES THE UPPER 6 INCHES OF THE ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AS REO'D BY ASTM DESIGNATION A153, CLASS C OR A164, TYPE G5.

EXPANSION

LOCATION	GIRDER NO.	А	В	С	D	NO. REQ'D.

FIXED



NO. DATE BY STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION STRUCTURE HEET BEARING DETAILS

SHOE BEARING - FIXED

7/8" DIA, DRILLED HOLES FOR 1 1/2" DIA. x 1'-10" LG. ANCHOR BOLTS. -

1/8" BEARING PAD

1 45/64" R. PLATE

1/2' /

1/4"

5/8'

- A.S.A. 250 3/4

SYM. ABOUT C

1 1/2"

END VIEW

A.S.A. 250

2 1/4"

9-1-94



BEARING NOTES

ALL MATERIAL EXCLUDING ANCHOR BOLTS AND PINTLES SHALL BE MADE OF A588 STEEL. ALL STRUCTURAL STEEL BEARING PLATES SHALL BE FLAT ROLLED STEEL PLATES WITH ALL SURFACES SMOOTH AND FREE FROM WARP AND ALL EDGES SMOOTH, STRAIGHT AND VERTICAL.

ALL PLATE CUTS SHALL BE MACHINE OR MACHINE FLAME CUTS.

MACHINE FINISH THE BOTTOM SURFACE ONLY OF PLATES SHOWN TO BE FINISHED. ALL MATERIAL EXCLUDING BRONZE PLATES, BEARING PADS AND ANCHOR PLATES SHALL BE PAID FOR AT THE UNIT PRICE BID FOR "HIGH STRENGTH STRUCTURAL STEEL"

ALL ANCHOR BOLTS TO BE $1^{1}/_4$ " × 1^{1} -3" LONG, SET FLUSH AND CAULKED WITH LEAD TO THE TOP OF PLATE 'C'. EXCESS LENGTH MAY BE FURNISHED, THREADED FOR SETTING AND THEN CUT OFF FLUSH.

CHAMFER TOP OF PINTLES $\frac{1}{8}$ ". DRILL HOLES FOR PINTLES IN PLATE "C" FOR DRIVING FIT. PROVIDE $\frac{1}{8}$ " THICK BEARING PAD SAME SIZE AS PLATE 'C'FOR EACH BEARING. ALL BEARINGS ARE SYMMETRICAL ABOUT & GIRDER AND & OF BEARING.

PINTLES SHALL CONFORM TO A.S.T.M. SPECIFICATION TYPE A449 STEEL, OR MATERIAL OF EQUIVALENT YEILD STRENGTH & ELONGATION.

ALL ANCHOR BOLTS SHALL CONFORM TO A.S.T.M. SPECIFICATION TYPE A36 STEEL.

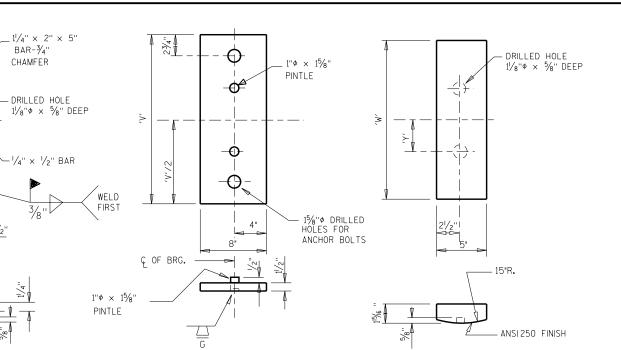


PLATE A

BRONZE PLATE

LUBRICATE TOP SURFACE ONLY. Ç OF

GIRDER

PLATE B

ANSI 250 FINISH

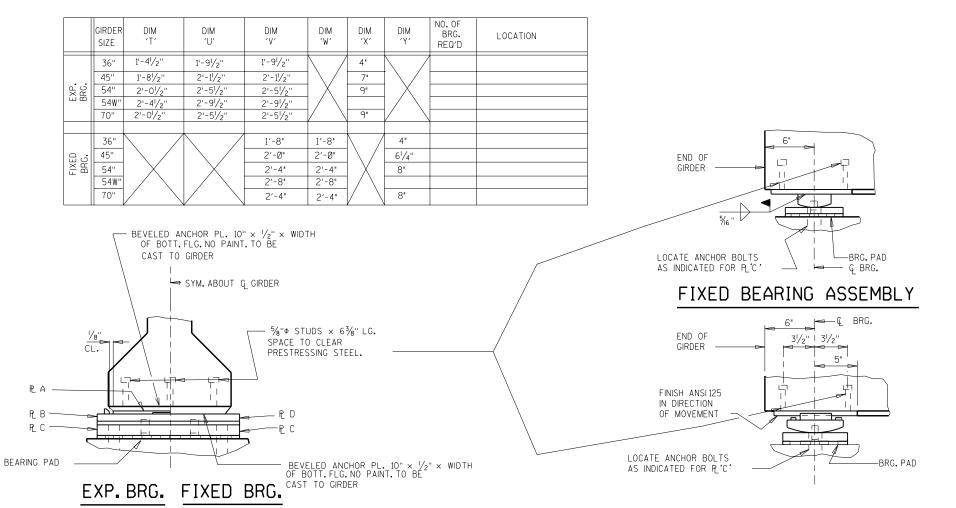
21/2"

BAR-¾"

CHAMFER

PLATE C

PLATE D



BEARING DETAILS

EXPANSION BEARING ASSEMBLY

LEGEND

- 1 FLEXIBLE RUBBER HOSE, MIN. 3/6" THICK WALL. I.D. TO SUIT DOWNSPOUT.
- (2) 10 GAGE ST. STL. CLAMP GRADE 304.
- 3 NEW 6" DOWNSPOUT PIPEING SHALL BE REINFORCED THERMOSETTING RESIN PIPING (RTRP) CONFORMING TO ASTM D2996 & 2310.
- 4 10 GAGE GALV. STEEL SADDLE.
- (5) %" o St. StL. THREADED ROD (GRADE 304) WITH ST. STL. HEX NUTS & WASHERS OR GALV. ROD.
- $\ \textcircled{6}\ ^{1\!\!/}_{2}\text{"}\phi$ X 8"LG. ST. STL. THREADED ROD (GRADE 304) WITH ST. STL. HEX NUTS & WASHERS.
- 7 6" LG. SPLIT RTRP STRIP. BOND TO #3, PLACE AT ALL CLAMP LOC'S.
- 8 BENT ANCHOR PL. 7 GAGE ST. STL. GRADE 304.
- (9) 1/2" ADHESIVE ANCHOR BOLTS. (ST. STL. BOLT & WASHER TYP.)
- 10 BREEZE "HI-TORQUE CLAMP #HTM-700 304 HALF HARD ST. STL. OR EQUIV. (4 REQ'D. PER EACH #1).
- 11) 7 GAGE ST. STL. GUSSET PL. WELD TO #2 & 8.
- 12) 3/8" X 1" LG. ST. STL. HEX HEAD BOLT WITH NUTLOCK WASHER.

DOWNSPOUT NOTES

CLEVIS, PIPE CLAMPS, HANGERS & MISS. ITEMS ARE INCLUDED IN BID ITEM "RTRP DOWNSPOUTS, 6".

TIGHTEN CLAMPS TO A TIGHT SLIP FIT (FOR TEMP. EXPANSION).

ALL ELBOWS TO BE LONG RADIUS TYPE.

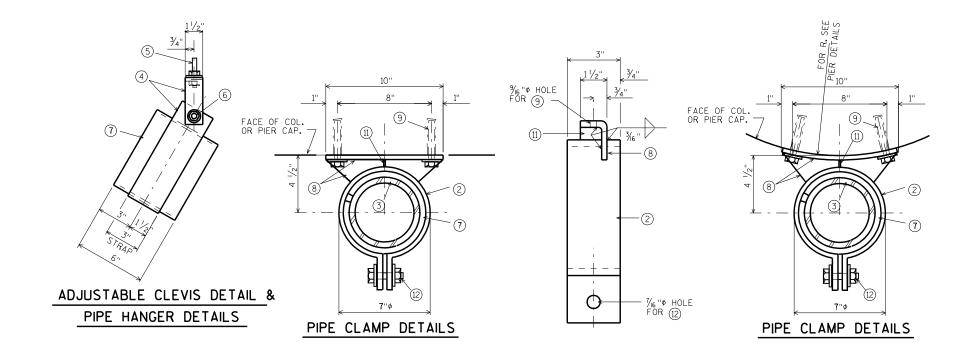
CONTRACTOR TO FIELD VERFIY ALL DEMINSIONS.

A CLAMP OF EQUAL STRENGTH MAY BE USED AS AN ALTERNATE.

RTRP DOWNSPOUT TO BE SUPPORTED AT 8'-0" MAX. SPACING USING ONE OF THE SHOWN BRACKETS, REPLACE DOWNSPOUT SYSTEM FROM EXIST. REDUCER TO TO NEW FLOOR DRAIN.

REMOVE EXISTING REDUCER & USE NEW FLEXIBLE CONNECTION TO CONNECT NEW DOWNSPOUT TO EXISTING DOWNSPOUT.ALL NEW SYSTEM AT PIER 9. INSTALL CLEAN-OUTS AT ELBOWS AS NEEDED.

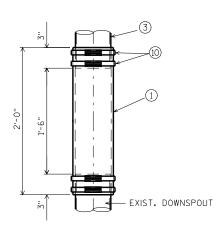
DRAIN WORK TO BE COMPLETED AS DIRECTED BY THE ENGINEER.



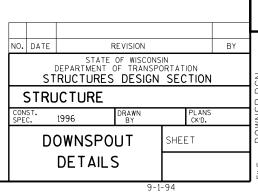
"/16 " P HOLE 7 3/8"

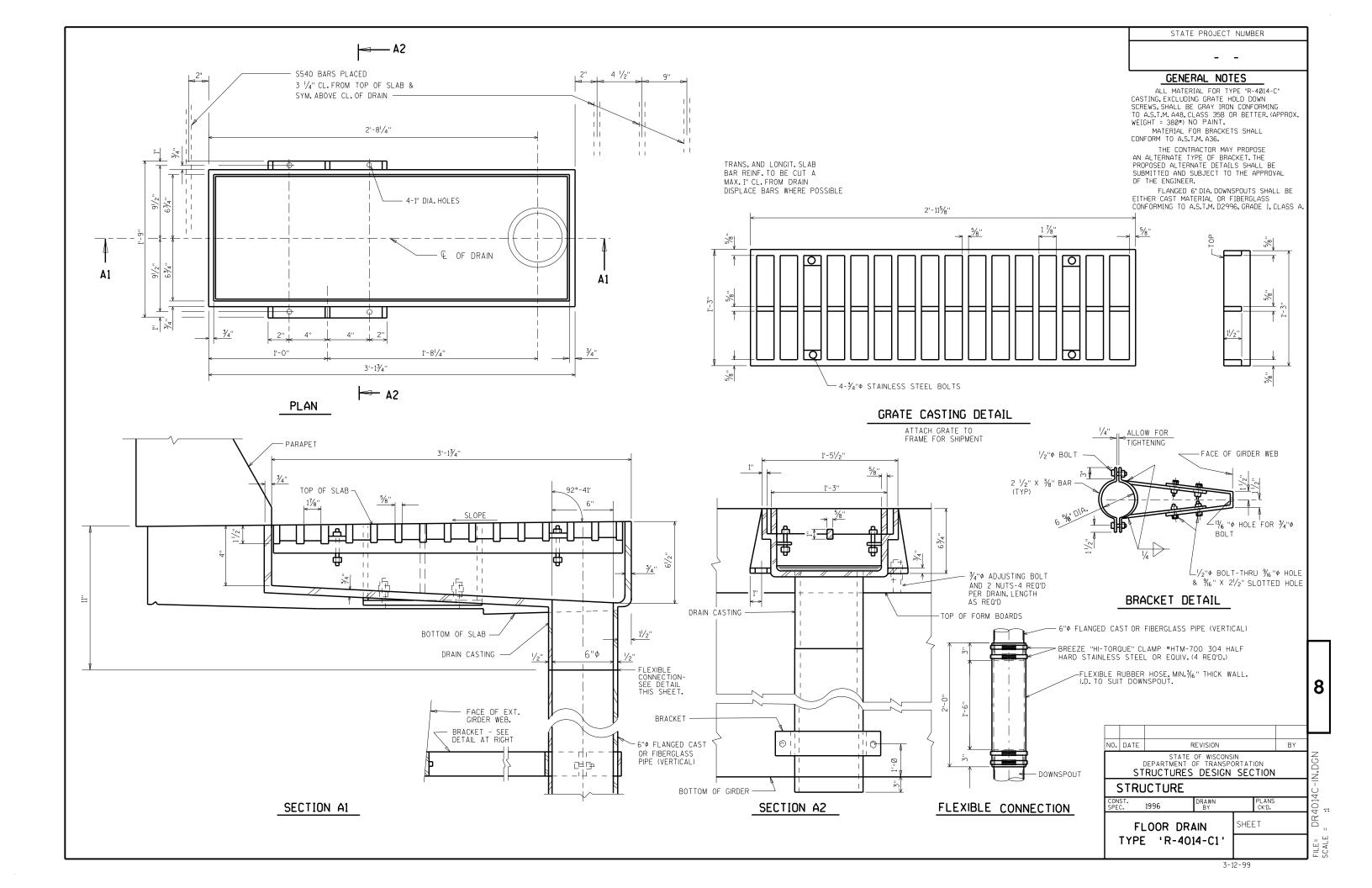
7" DIA. SADDLE

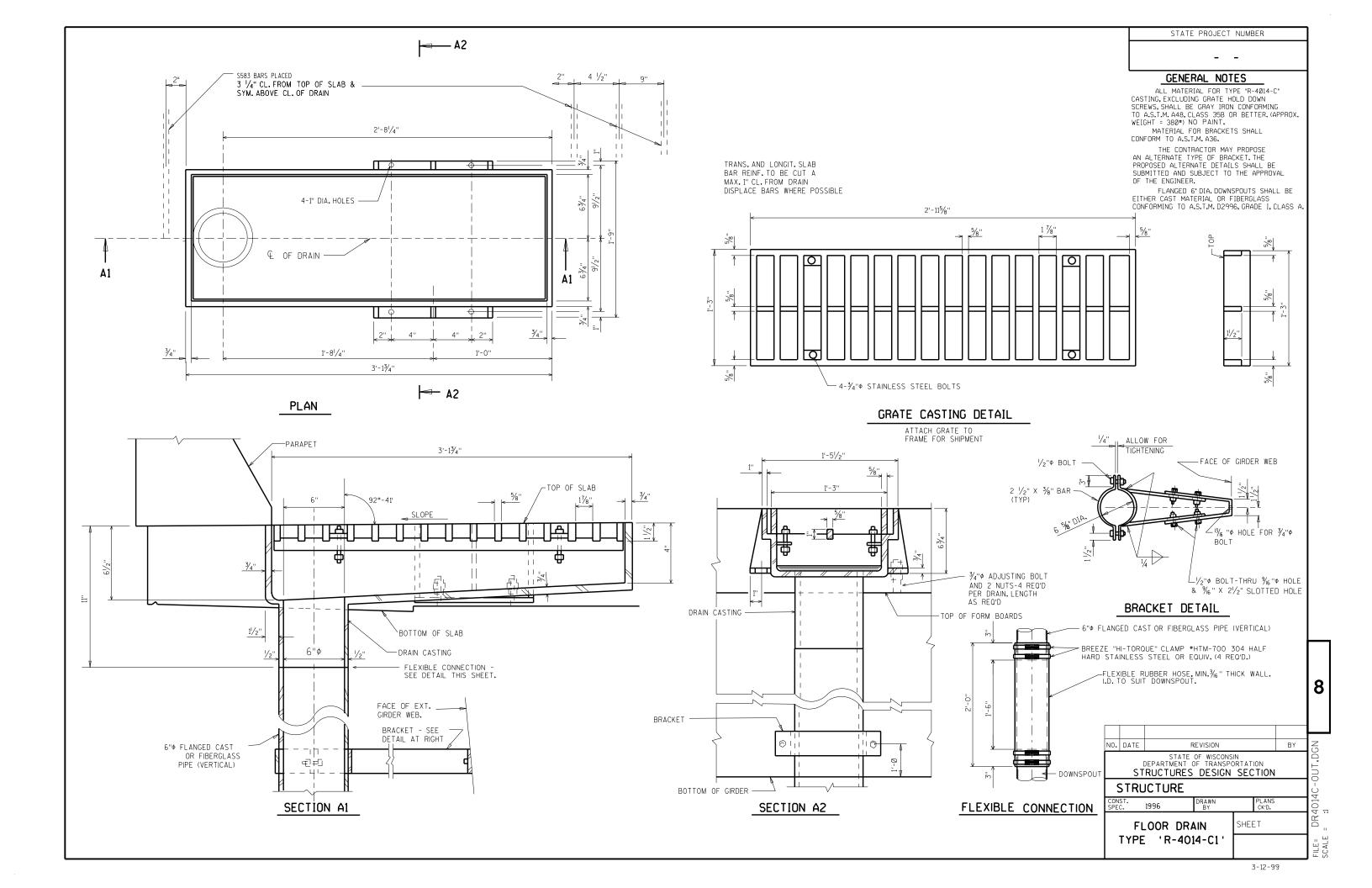
% "Φ HOLE-

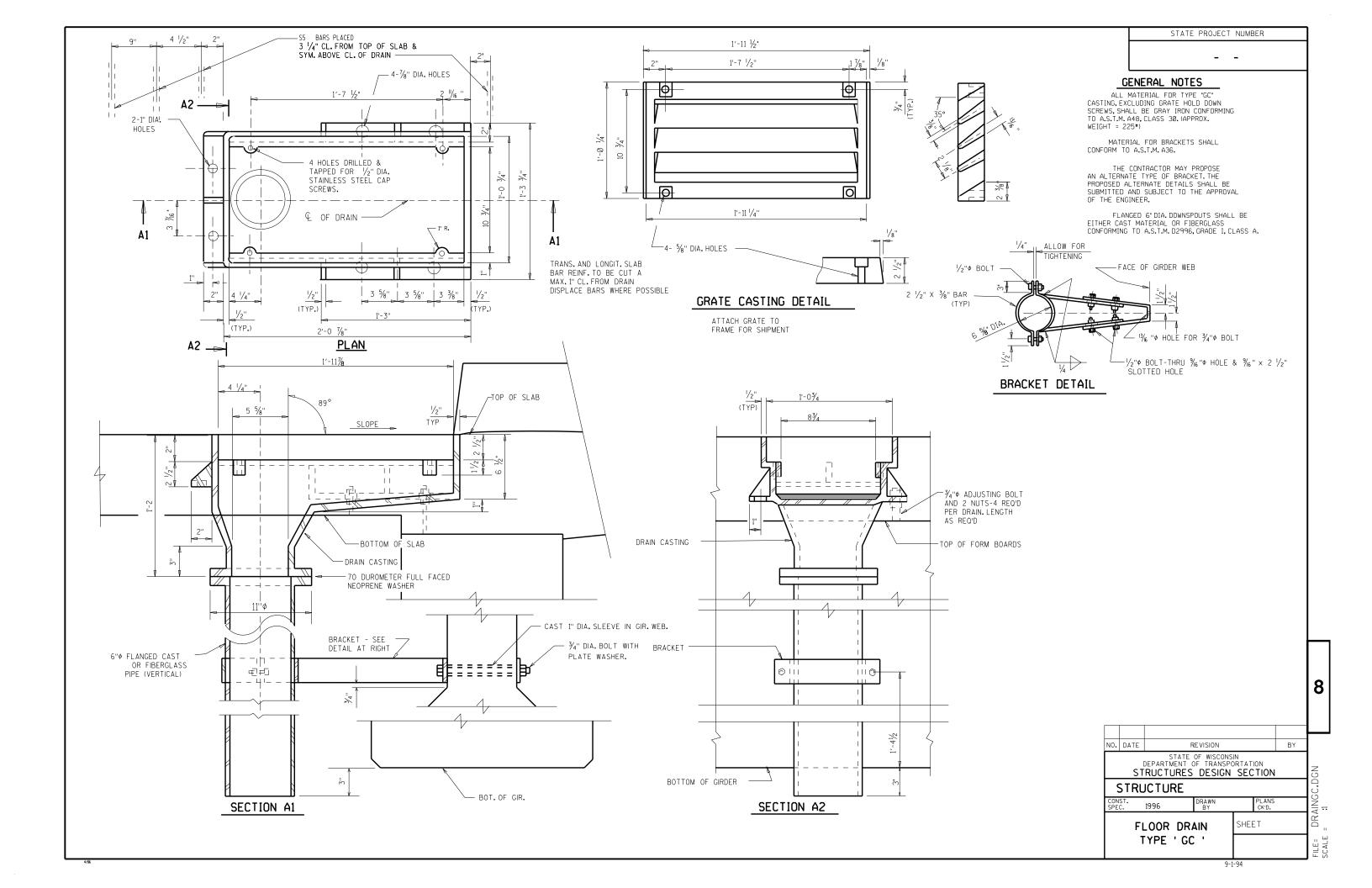


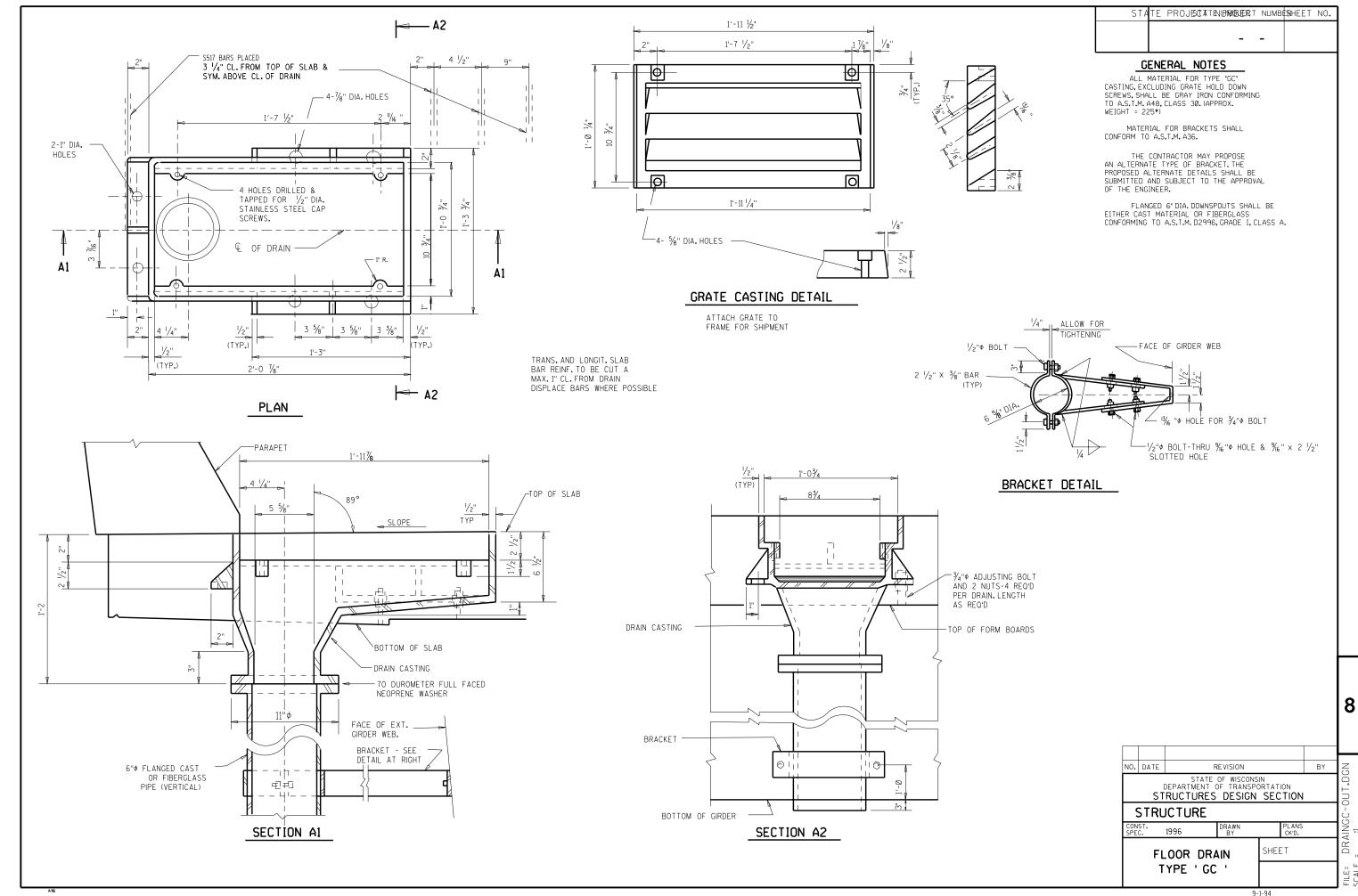
FLEXIBLE CONNECTION

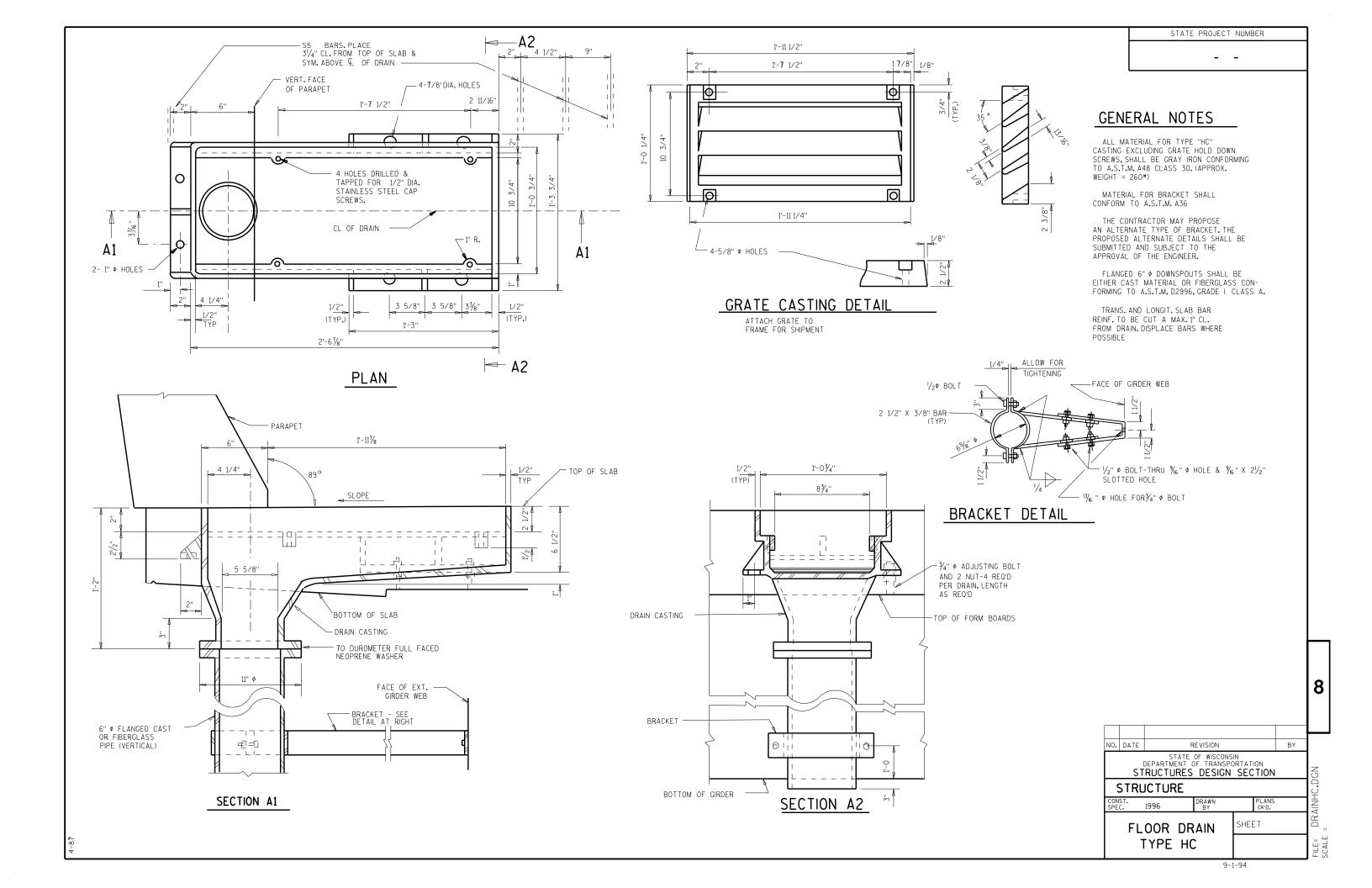


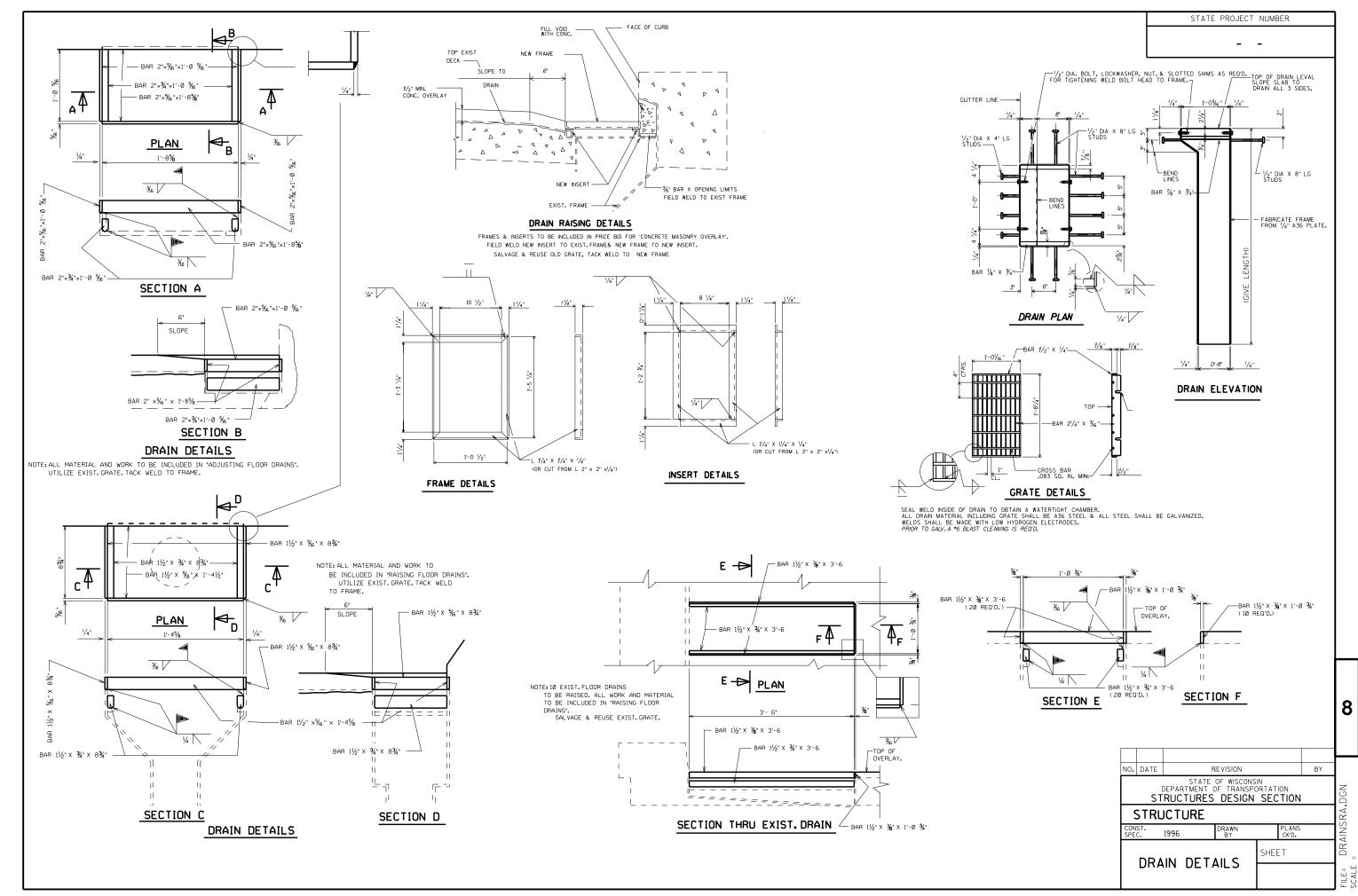










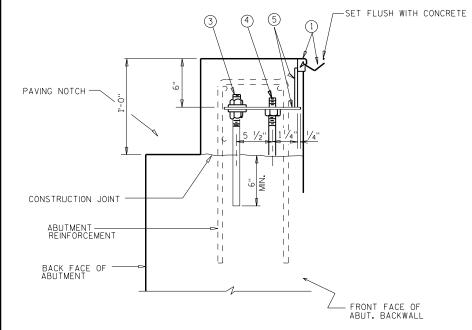


- 1. NEOPRENE STRIP SEAL (_ INCH) & STEEL EXTRUSIONS.

 SET JOINT OPENING AT 13/4" WHEN EXPANSION LENGTH

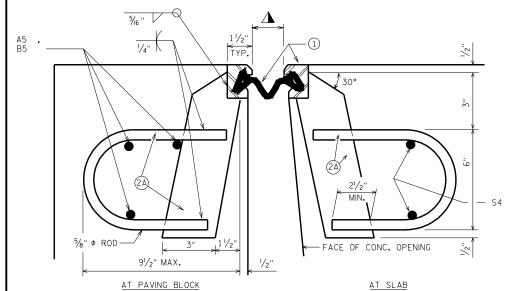
 SET JOINT OPENING AT 13/4" WHEN EXPANSION LENGTH 230'-0". WHEN EXPANSION LENGTH > 230'-0", PREPARE A TEMPERATURE TABLE SHOWING JOINT OPENINGS AT 85° F, 40° F, & -5° F.
- 2. STUDS 5%" ¢ X 63%" LONG AT 6" ALTERNATE CENTERS. WELD TO EXTRUSIONS & BEND AS SHOWN AFTER WELDING.
- 2A. 3" \times $^{1}\!/_{2}$ " Anchor plate with $^{5}\!/_{8}$ " ϕ Rod (or alternate strip seal anchor). Weld rod to anchor plate, weld anchor plate to *1 at 1'-6" ctrs. between girders.
- 4. 3/4" THREADED ROD WITH NUT. TACK WELD NUT TO NO.5.
- 5. FABRICATE SUPPORT FROM 3" X $/\!\!/_2$ " BAR AS SHOWN OR EQUIVALENT, ONE PER GIRDER PER SIDE, FIELD OR SHOP WELD TO NO.1. IF FIELD WELDED, COVER WELDED AREAS WITH EPOXYCOATING MATERIAL. PROVIDE $1\,{}^{\prime}\!/_2$ " ϕ HOLE FOR NO.3 & 1" ϕ HOLE FOR NO.4.

LEGEND



SECTION THRU JOINT AT ABUTMENT

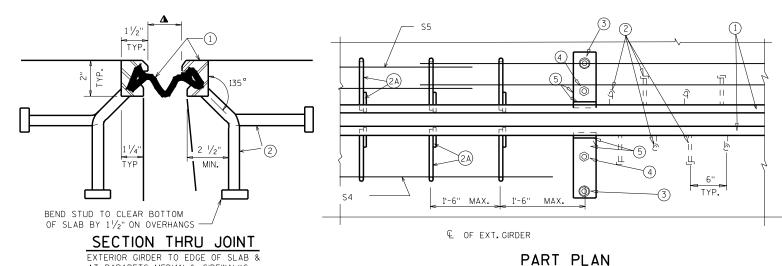
NORMAL TO & SUBSTRUCTURE



SECTION THRU JOINT

ROADWAY TRAFFIC AREA BETWEEN EXTERIOR GIRDERS.

AT PARAPETS, MEDIAN & SIDEWALKS



GENERAL NOTES

ONE FIELD SPLICE PERMITTED IN STEEL EXTRUSIONS. IF USED, DETAILS SHALL BE SUBMITTED FOR APROVAL. NO SPLICING PERMITTED IN NEOPRENE STRIP SEAL.

AFTER FABRICATION, BUT BEFORE SHIPMENT, STRAIGHTEN STEEL EXTRUSIONS SUCH THAT THEY SHALL BE FREE FROM WARP, TWIST & SWEEP.

FABRICATOR SHALL PROVIDE MEANS OF KEEPING GALVANIZED EXTRUSIONS CLEAN & SMOOTH DURING SHIPMENT AND PRIOR TO APPLYING LUBRICANT ADHESIVE FOR NEOPRENE GLAND INSTALLATION.

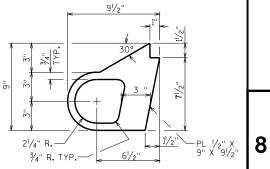
SANDBLAST PLATES & EXTRUSIONS AFTER FABRICATION IN ACCORDANCE WITH SSPC SP. #6 "COMERCIAL BLAST CLEANING". AFTER BLAST CLEANING THE PLATES & EXTRUSIONS SHALL BE HOT DIPPED GALVANIZED.

ANCHOR SYSTEM *8 & *9 SHALL CONFORM TO ASTM A307 & SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 CLASS C & D.

STRIP SEAL EXPANSION JOINT ASSEMBLY, INCLUDING ANCHOR STUDS & HARDWARE WILL BE PAID FOR AT THE LUMP SUM PRICE BID FOR "EXPANSION DEVICE".

BLOCK OUT CONCRETE 2" EACH SIDE OF JOINT OPENING.

 \square JOINT OPENING DIM. ALONG SKEW PLUS 1/2".

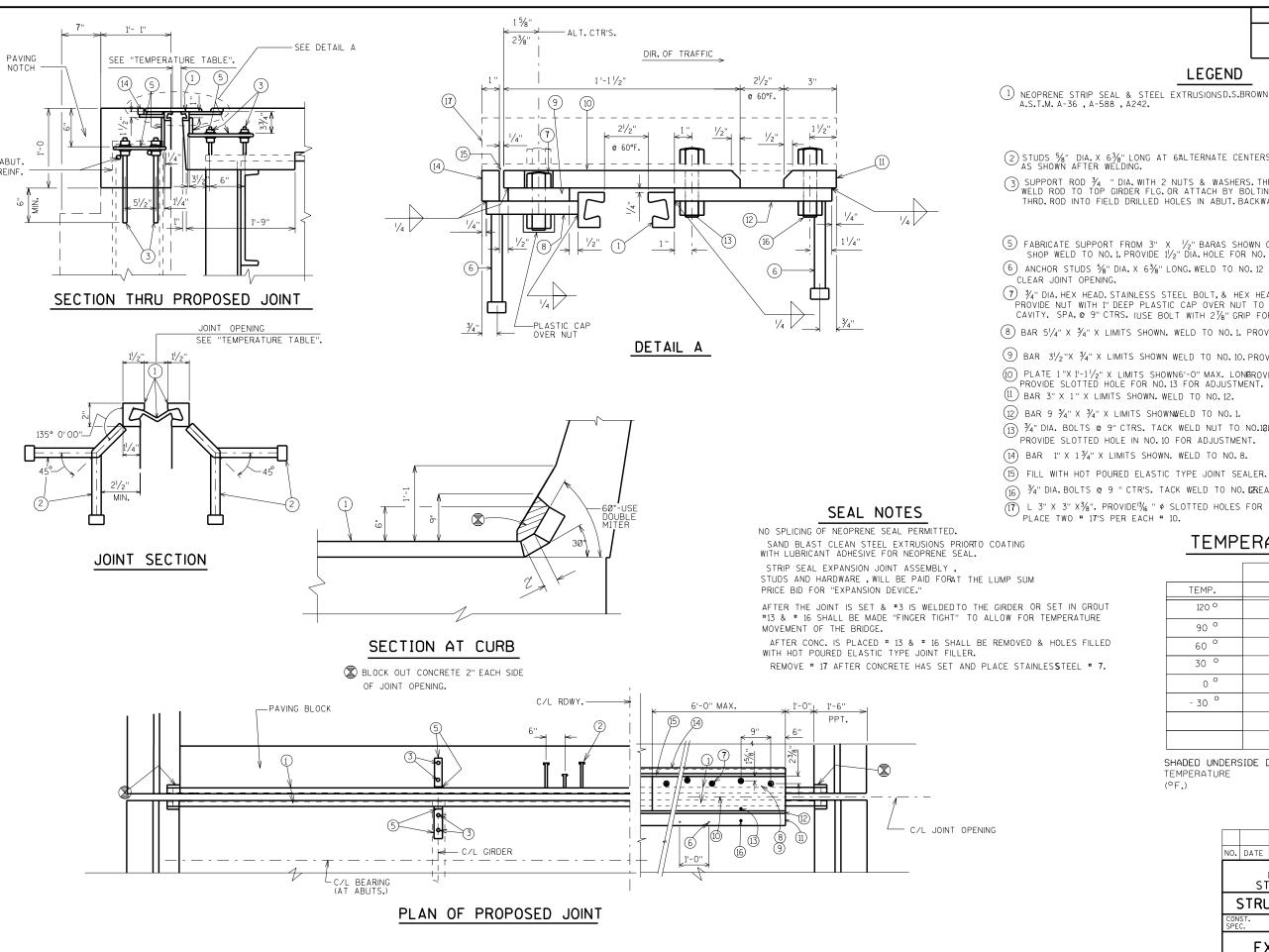


ALTERNATE STRIP SEAL ANCHOR

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EXPANSION SHEET						/ <u></u>	
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STATE PROJECT NUMBER

- 1 NEOPRENE STRIP SEAL & STEEL EXTRUSIONSD.S.BROWN SS-600EXTRUSIONS TO BE EITHER
- $\ \ \,$ STUDS $\ \ \,$ DIA.X 6% LONG AT 6ALTERNATE CENTERS. WELD TO EXTRUSION & BEND AS SHOWN AFTER WELDING.
- $\ \ \,$ Support rod $\ \ \,$ " dia. With 2 nuts & Washers. Thrd. One end 4AT super side of jt. Weld rod to top girder flg. or attach by bolting thru flg. At about THRD. ROD INTO FIELD DRILLED HOLES IN ABUT. BACKWALL AS SHOWN.
- 5 FABRICATE SUPPORT FROM 3" X 1/2" BARAS SHOWN OR EQUIVALENT, ONE PER GIRDER. SHOP WELD TO NO. 1. PROVIDE 11/2" DIA. HOLE FOR NO. 3 & 1" DMGLE FOR NO. 4.
- (6) ANCHOR STUDS 5%" DIA. X 63%" LONG, WELD TO NO. 12 & NO. 8 AS SHOWN, BEND TO
- 7 $\frac{1}{4}$ " DIA. HEX HEAD. STAINLESS STEEL BOLT, & HEX HEAD NUT. WELD NUT TO NO. 8. PROVIDE NUT WITH 1" DEEP PLASTIC CAP OVER NUT TO KEEP CONC. FROM ENTERING BOLT CAVITY. SPA. @ 9" CTRS. (USE BOLT WITH $2\frac{7}{8}$ " GRIP FOR INSTALLATION)
- 8 BAR 51/4" X 3/4" X LIMITS SHOWN. WELD TO NO. 1. PROVIDE HOLES FOR NO. 7.
- 9 BAR 31/2"X 3/4" X LIMITS SHOWN WELD TO NO. 10. PROVIDE HOLES FOR NO. 7.
- (0) PLATE 1"X 1'-1½" X LIMITS SHOWN6'-0" MAX. LONGROVIDE 15%" \$\phi\$ HOLES FOR NO. 7. PROVIDE SLOTTED HOLE FOR NO. 13 FOR ADJUSTMENT.

- (3) 3/4" DIA. BOLTS @ 9" CTRS. TACK WELD NUT TO NO.10REASE FOR EASY REMOVAL. PROVIDE SLOTTED HOLE IN NO. 10 FOR ADJUSTMENT.

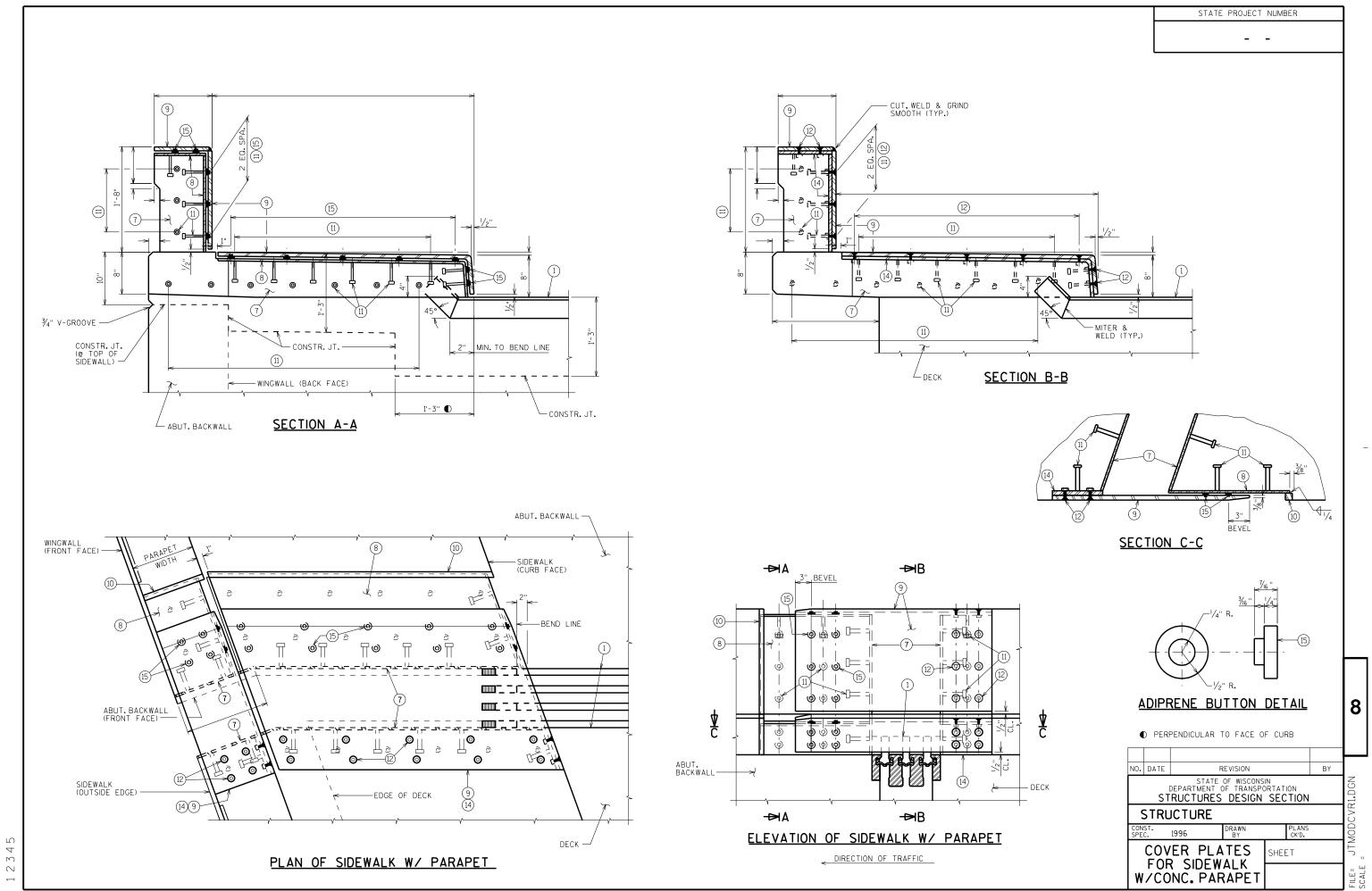
- $\ensuremath{\text{(16)}}$ $\ensuremath{\text{34}}\xspace"$ DIA.BOLTS @ 9 " CTR'S. TACK WELD TO NO. CREASE FOR EASY REMOVAL.
- (17) L 3" X 3" X $\frac{3}{8}$ ". PROVIDE $\frac{13}{16}$ " ϕ SLOTTED HOLES FOR # 13 & # 16.

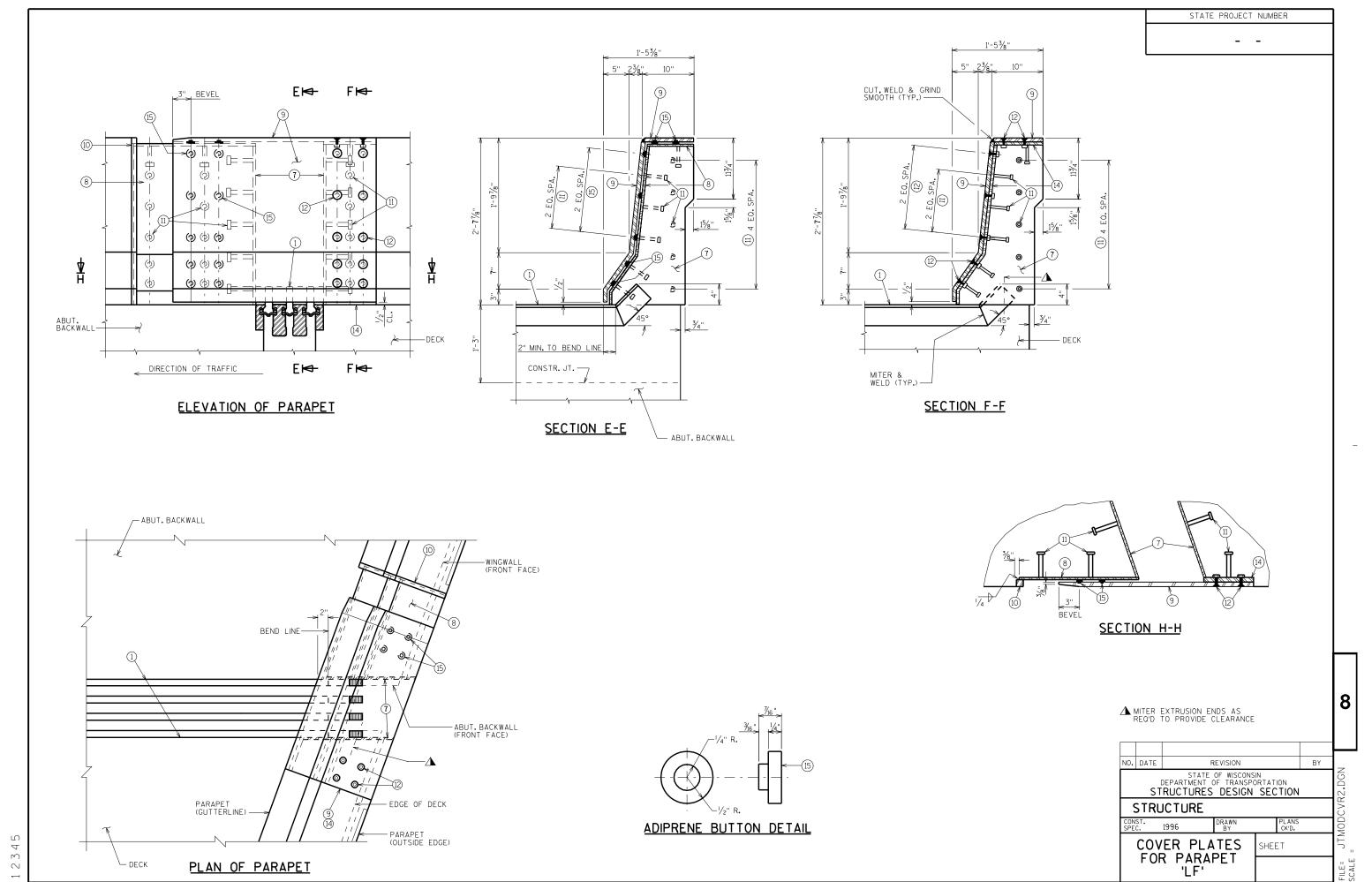
TEMPERATURE TABLE

	JOINT OPENING		
TEMP.			
120 °			
90 °			
60 °			
30 °			
0 °			
- 30 °			

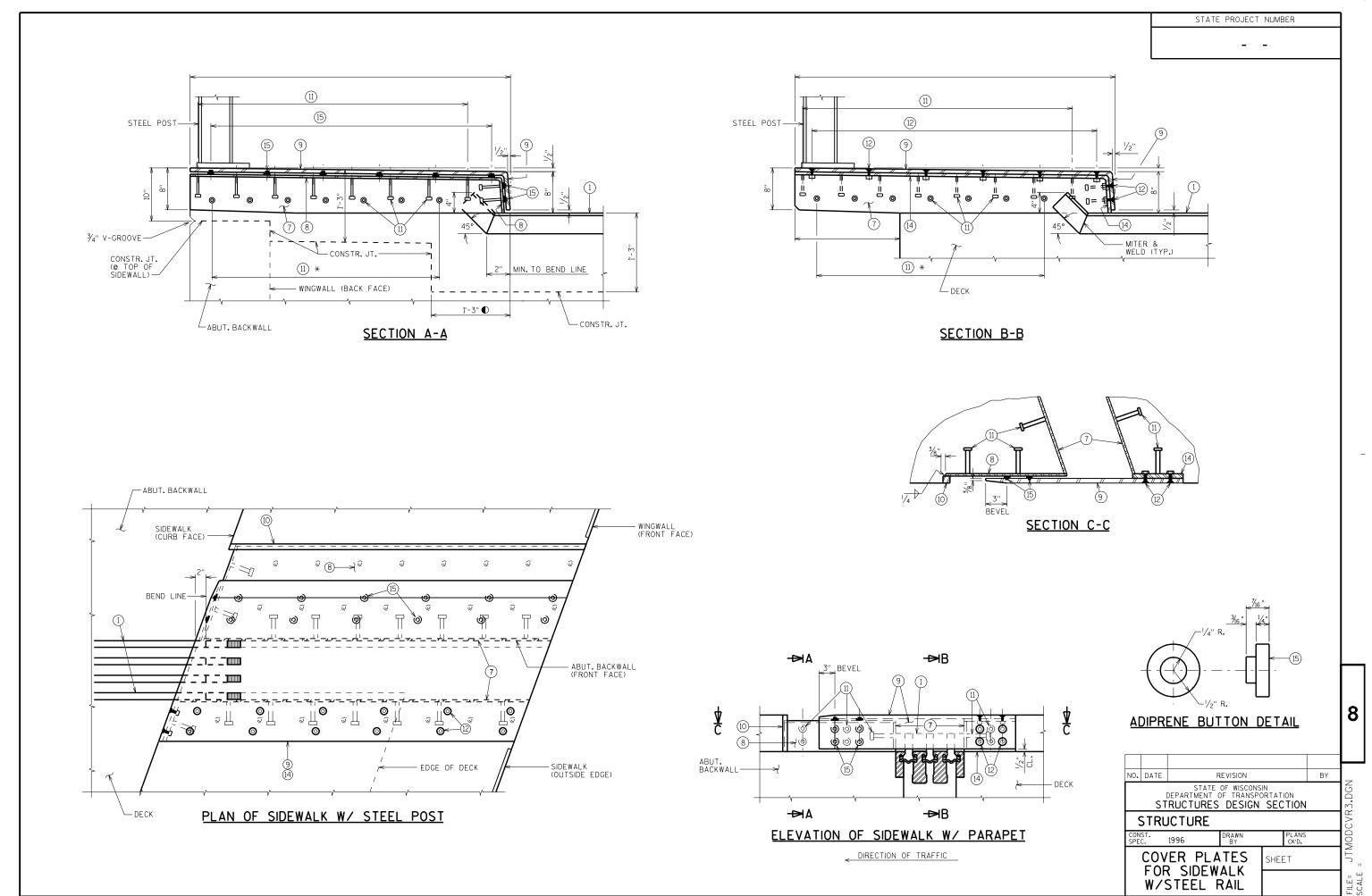
SHADED UNDERSIDE DECK TEMPERATURE

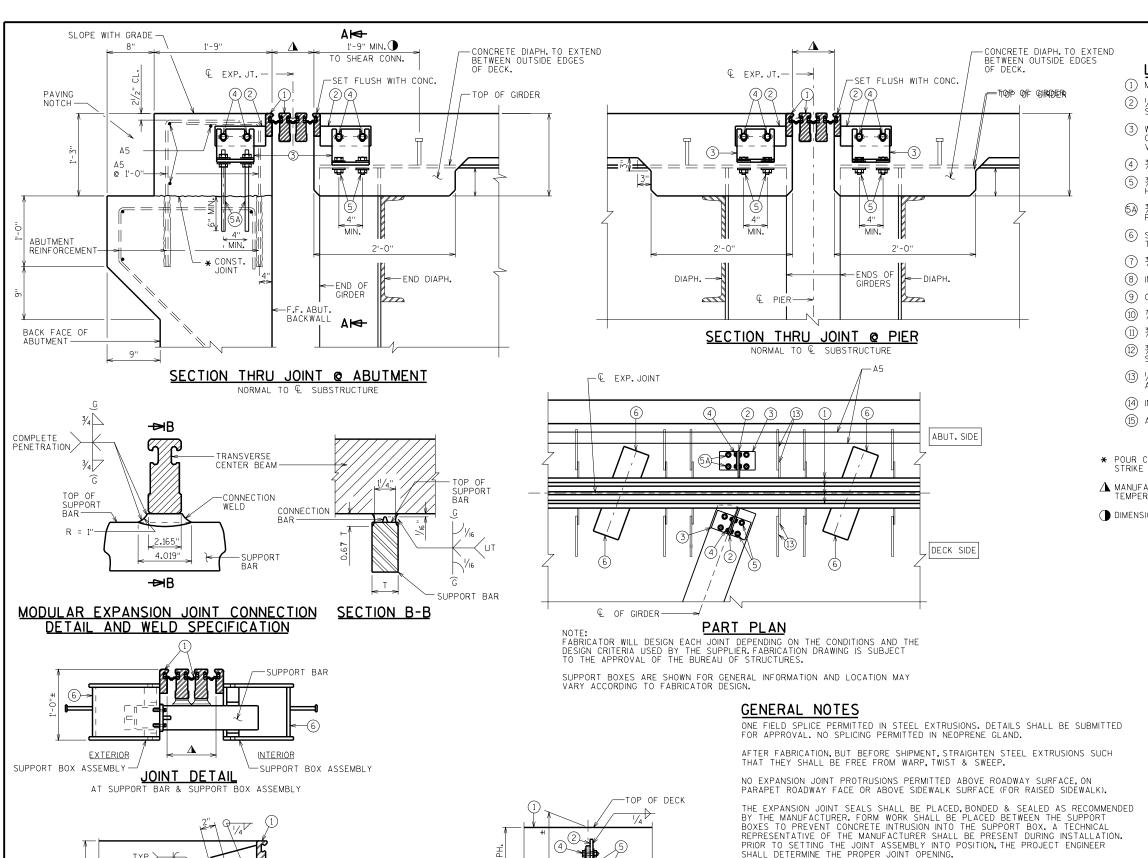
> NO. DATE STATE OF WISCONSIN DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN SECTION STRUCTURE SHEET **EXPANSION** JOINT





-02





END DIAPH. — STEEL GIRDER -

SECTION A-A

ANCHORAGE DETAIL

PLACE ADJACENT TO SUPPORT BOXES IN PAVING BLOCK @ ABUT. & IN DECK @ CONC. DIAPH.

LEGEND

- (1) MODULAR EXPANSION JOINT DEVICE.
- $\begin{tabular}{ll} \begin{tabular}{ll} \beg$
- 3 WT 6 X 29 (OR EQUIVALENT BUILT UP T-SECTION), ONE PER GIRDER, PROVIDE 2 1" X 3" MIN, SLOTTED HOLES PLACED VERTICALLY IN WEB OF WT FOR BOLTS NO. 4.
- (4) 3/4" \$\phi\$ HIGH STRENGTH BOLTS WITH NUTS & WASHERS. (A325 GALV.)

STATE PROJECT NUMBER

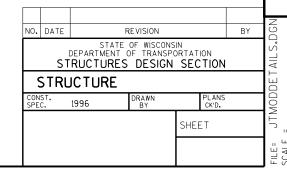
- (5) 3/4" \(\psi\$ HIGH STRENGTH BOLTS WITH NUTS & WASHERS. FIELD DRILL HOLES IN GIRDER TOP FLANGE. (A325 GALV.)

- 7) 3/8" BULKHEAD PLATE. WELD TO NO. 1, NO. 8 AND NO. 14.
- 8 INSIDE PLATE. FABRICATE FROM 3/8" PLATE.
- 9 OUTSIDE PLATE, FABRICATE FROM 5%" PLATE.
- (10) 78" SQUARE BAR. WELD TO NO. 8 AS SHOWN.
- (11) $\frac{3}{4}$ " ϕ X 4" LONG STUDS. WELD TO NO. 8 , NO. 7 & NO. 14 AS SHOWN.
- 2 $\cancel{3}\!\!/_4$ " ϕ X 2" STAINLESS STEEL FLAT CTSK. SLOTTED HEAD CAP SCREWS. RECESS $/\!\!/_6$ " BELOW PLATE SURFACE.
- (3) 1/2" PLATE WITH 5%" \$ LOOP ANCHOR FABRICATED AS SHOWN, SPACED AT MANUFACTURER'S SPEC.
- (14) INSIDE PLATE. FABRICATE FROM 5/8" PLATE
- (15) ADIPRENE BUTTON, SEE DETAIL, SET IN OUTSIDE PLATE.
- * POUR CONC. ABOVE THIS JOINT AFTER SUPERSTRUCTURE CONC. IS IN PLACE. STRIKE OFF & LEAVE ROUGH.
- ⚠ MANUFACTURER'S RECOMMENDED JOINT OPENING BASED ON THE TEMPERATURE ON THE DAY OF PLACEMENT PER TEMPERATURE TABLE.
- DIMENSION IS PARALLEL TO & GIRDER.

TEMP. TABLE

TEMPERATURE TABLE FOR SETTING JOINT OPENINGS TO BE DETERMINED BY JOINT MANUFACTURER WITH THE FOLLOWING DESIGN DATA:

- 1. IN. OF MOVEMENT PER 10° F 2. MEDIAN TEMPERATURE OF 45° F
- 3. TEMP. RANGE IN TABLE FROM (- 5° F) TO (+ 95°F)
- A TABLE OF JOINT OPENINGS BASED ON ABOVE DATA SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.



EXPANSION JOINT EXTRUSIONS SHALL BE FABRICATED TO CONFORM TO ROADWAY CROWN & GRADE. FABRICATOR SHALL PROVIDE MEANS OF KEEPING GALVANIZED EXTRUSIONS CLEAN & SMOOTH DURING SHIPMENT AND PRIOR TO APPLYING LUBRICANT ADHESIVE FOR NEOPRENE GLAND INSTALLATION.

SANDBLAST BARS, PLASS, WT-SECTION, ANCHORAGE LOOP. & EXTRUSIONS AFTER ROADBLAST BARS, PLASS WT-SECTION, ANCHORAGE LOOP. & EXTRUSIONS AFTER

SANDBLAST BARS, PLATES, WT-SECTION, ANCHORAGE LOOP, & EXTRUSIONS AFTER FABRICATION IN ACCORDANCE WITH SSPC SP. #6 "COMMERCIAL BLAST CLEANING". AFTER BLAST CLEANING, THIS ASSEMBLY SHALL BE HOT DIPPED GALVANIZED.

COST OF FURNISHING & PLACING OF THE EXPANSION JOINTS COMPLETE WITH PARAPET PLATES & SIDEWALK PLATES SHALL BE PAID FOR UNDER THE PRICE BID FOR "MODULAR EXPANSION DEVICE, STRUCTURE B- - ".

BAR STEEL REINF.IN DECK AND CONC. DIAPHRAGM SHALL BE RESPACED AS NECESSARY TO ALLOW PLACEMENT OF JOINT ASSEMBLY.

